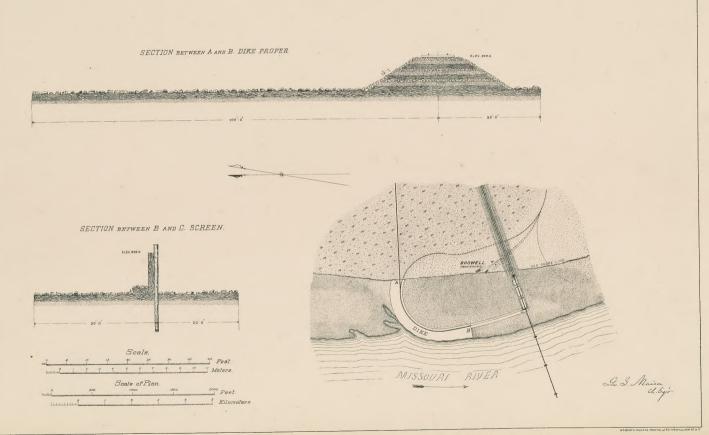
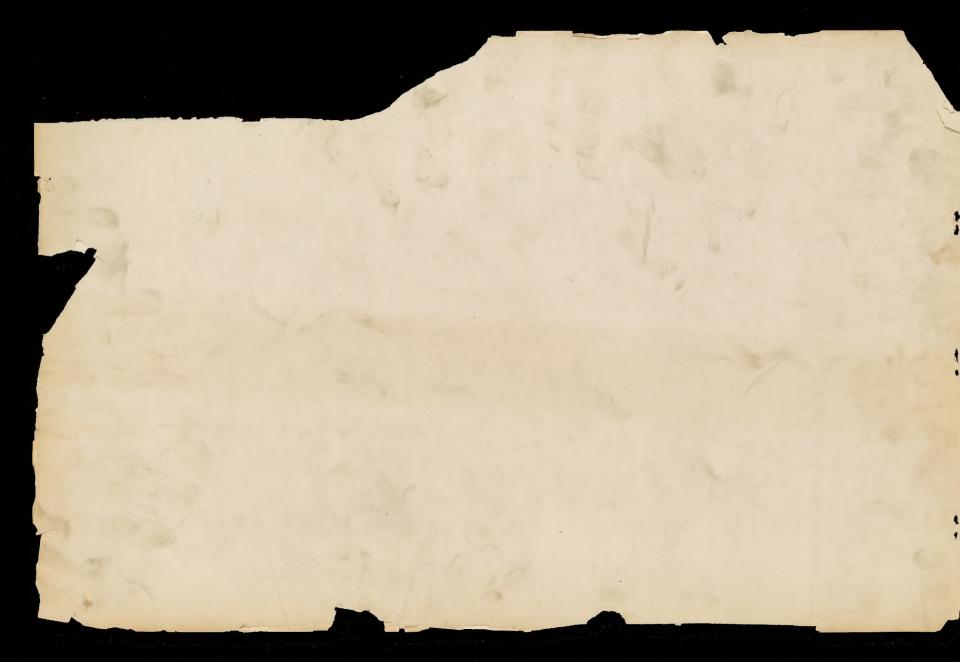
# C.B.R.Q.H.R. RULO BRIDGE

DETAILS OF DIKE.





GEO. S. MORISON.

Chief Engineer.

The Rookery.

# THE RULO BRIDGE.

# A REPORT

To CHARLES E. PERKINS, President of the Chicago, Burlington & Quincy Railroad,

3Y

GEORGE S. MORISON, Chief Engineer of the Rulo Bridge.

#### PLATES. SUBJECTS. APPENDICES. 13. General Elevation and Plan of Through Span. r. General Map. 14. Through Span; Panel Point o. 2. General Elevation, Plan, Profile and Alignment. A. List of Engineers, Employees and Contractors . . . 13 15. Through Span; Panel Points 1 and 2. 3. Profile showing Stratification on Bridge Line. 16. Through Span; Panel Points 3, 4 and 5. 4- Pier I. 17. Through Span; Panel Points 6 and 7. 5. Pier II. IV. Superstructure . . . . . . . . . . . . . . . 9 18. Through Span; End Elevation. 6. Pier III. 19. Deck Span; Elevation and Plan. 7. Pier IV. 20. Deck Span; Details. G. Tests of full-sized Steel Eye bars . . . . . . . 30 8. Caissons II, and III. 21. Supporting Towers. q. Diagram showing Rate of Progress in Sinking Caissons. 22. Strain Sheets. 10. Clay Hoist. 23. Details of Dike. 11. Record of Water Stage. 12. Approach Pier.

CHICAGO, MAY 1, 1890.

CHARLES E. PERKINS, ESQ.,

President Chicago, Burlington & Quincy Railroad.

Dear Sir:-

I submit the following final report in relation to the construction of the bridge across the Missouri river at Rulo, Nebraska.

Yours truly,

GEO. S. MORISON,

Chief Engineer Rulo Bridge.

### THE RULO BRIDGE.

#### Ī.

### PRELIMINARY NARRATIVE.

Before the completion of the Plattsmouth Bridge in 1880, the importance of a lower crossing of the Missouri to accommodate the Southern lines of your Nebraska system of railroad, became apparent, and I was instructed by you to make an examination with a view to locating such a bridge near the mouth of the Little Nemaha. This examination was made in the season of 1880-'81 and resulted in the selection of a location near the little village of Aspinwall about two miles below the mouth of the Little Nemaha. A bridge built here would have made a direct connection between the railroad leading westerly from Nemaha City to Beatrice and the branch on the east side of the river from Corning to Villica.

Subsequently the acquisition of the Hannibal & St. Joseph R. R. by your company made a more southerly crossing desirable and the fact that the Atchison & Nebraska R. R. (a portion of your Nebraska System) followed down the valley of the Great Nemaha indicated that a location at Rulo near the mouth of the Great Nemaha would have decided commercial advantages over the Aspinwall location.

The season of 1883-'84 was therefore devoted to examinations and surveys in the neighborhood of the mouth of the Great Nemaha.

The beginning of the work may be fixed as September 13th, 1883, when Mr. B. L. Crosby the Resident Engineer arrived at Rulo. The location at Rulo was fixed by me and borings were actually begun one week after Mr. Crosby's arrival.

These borings showed a state of affairs quite unlike that usually found in

the Missouri River, there being no rock within any reasonable depth, but a stiff bed of blue clay, of an average thickness of about 15 feet was found under the alluvial sand, this clay resting on a bed of coarse sand and gravel of varying thickness, which itself rested on a bed of clay, the surface of which was nearly level and which from its stratified character was found to be more truly a shale than a clay. These borings showed that though the bridge when built would be of a satisfactory character, the cost of the foundations would be exceptionally large. It was evidently expedient to examine other points in the same neighborhood.

Borings were accordingly made in the spring and summer of 1884 at White Cloud to miles below Rulo and at Arago 10 miles above Rulo, these being the nearest points at which the general topography of the country indicated that the construction of a bridge would be feasible. The borings at White Cloud gave better results than at Rulo, as rock bottom was reached, but at Arago, on the east side of the river, hard material was found only at a depth of 123 feet from top of sand bar, this material being a very soft sand stone. The difficulties in approaching a bridge at either of these points were so great that Rulo was selected as decidedly the best location.

The width of the river at ordinary high water stage at Rulo was about 1600 feet, the channel being next to the west shore. This width being greater than the width required to pass the river, it was determined to reduce this width to about 1100 feet by the construction of a dike above the bridge. Construction of this dike was authorized in October 1884, work was begun on the track leading to this dike October 22d; the dike itself was begun on the 4th of December and completed May 7th, 1885, and an extension in the form of a permeable screen made in the following May and June.

Authority for the construction of the bridge was obtained from the general government in 1884 by an Act which became a law June 18th, 1884. This Act is printed in full in Appendix B.

The location of the bridge had been definitely fixed at the time that Rulo was selected in preference to any other place. The character of the bridge was, however, not fixed at this time. It was evident to me from the beginning that the only proper structure was a high bridge without a draw, the western approach to which would run nearly due west connecting with the Atchison & Nebraska Railroad in the Nemaha valley. In this opinion I had the hearty support of Mr. R. J. McClure, Chief Engineer of the Chicago, Burlington & Quincy R. R. and Mr. J. F. Barnard, then General Manager of the Hannibal & St. Joseph R. R. who really had been the first to call attention to the merits of Rulo as a place for crossing the Missouri River.

There was, on the other hand, a decided demand by some of the operating of the company for a low bridge, the west approach to connect with the old track of the Atchison & Nebraska R. R. in front of the town of Rulo, which ran south along the Missouri bottom till it reached the Nemaha valley. The merits of a high bridge scheme was its simplicity, a less cost of maintenance of the bridge and the fact that it shortened the through distance two miles. The only advantage of the low bridge scheme was that it avoided the deep cut west of Rulo, and a careful estimate showed that a low bridge would be the more expensive of the two. The difference in opinion prevented an early determination of the plan of bridge and did much to render the cost of real estate on the west side of the river unreasonably large.

In 1885 and before the character of bridge had been determined, I asked for authority to put in the foundation of Pier I (the eastern pier) with a view of determining more fully the character of the material on which the piers would rest. This authority was granted and work was actually begun December 3, 1885, this foundation being finished in the following April.

The character of the bridge was finally determined and the plans were submitted to the Secretary of War for approval July 19, 1886. No effort was made to get this approval quickly, and it was not finally received until February 25, 1887, work, however, had meanwhile been in progress.

In May 1886 authority was given to continue the construction of the bridge in earnest and the work was prosecuted from this time forward without delay under the direct charge of Mr. B. L. Crosby as Resident Engineer.

The winter of 1886-87 was unfavorable for work, it being one in which the ice formed and broke up several times, this causing some delay and increasing the cost of the work.

The last span of the bridge was swung September 24, 1887, and on the afternoon of October 2d the first locomotive crossed the bridge, and it was opened to traffic immediately thereafter.

The great cut on the west approach to the bridge was, however, not yet completed, and for nearly two years the traffic crossing the bridge was taken over the old line between Rulo and Rulo Y.

On June 3, 1889 the excavation for the great cut was completed, though the track remained to be laid and much ditching to be done; on July 14th the first train passed through the great cut. The ballasting of the track through the great cut was completed September 8th.

On November 1, 1889, the bridge with its approaches was turned over to the operating department as a completed structure.

#### H.

#### GENERAL DESCRIPTION.

The Rulo bridge is a single track railroad bridge. It consists of three channel spans each 375 feet long between centers of end pins, resting on four piers of granite masonry (numbered from east to west), at each end of which are three 125 feet deck spans, the spans being separated by iron towers 25 feet long, making the length of the iron structure at each end of the channel spans 425 feet. The end pins are placed 4 ft. 6 in. between centers over Piers II and III, and 3 ft. 6 in. between centers over Piers I and IIV, and the end pin of the deck span is 1 ft. ½ in. from the back of the bolster, thus making the entire length of the whole structure from end to end of iron or steel work, 1993 feet 1 inch.

The bridge is built on a grade of 0.4 per cent. (21.12 ft. per mile) ascending westward. The clearance at the center of the east span was 53 feet above the water of April 14, 1884, and that at the center of the west span 56 feet above the same high water, this stage of high water being about six feet below the highest water observed except that of 1881; these clearances were both accepted in the approval of the plans by the Secretary of War. The actual clearances above the extraordinary flood of 1881, so far as this can be determined, are 43.5 and 46.5 feet respectively. Since the construction of this bridge, a Standard High Water has been established by the Missouri River Commission at this place (circular of April 24, 1889); the clearances above this high water are 5.0.8 and 47.8 feet respectively.

The east approach is 15.220 feet long from a connection with the track built by the St. Joseph & Nebraska R. R., in Section 36, T. 61 N., R. 40 W., to the end of the iron work, the maximum grade on this approach being 0.5 per cent. (26.4 ft. per mile.)

The west approach is 19,260 feet long from the west end of the iron work to the connection with the Atchison & Nebraska R. R., the maximum grades being 0.4 per cent. (21.12 ft. per mile) in each direction, excepting a short piece of one per cent. put in temporarily at the connection.

Besides the two approaches proper, a third approach, called the Atchison connection, was built connecting the west approach with the old line of the

Atchison & Nebraska R. R., this connection being 1.36 miles long, with maximum grades of 1.5 per cent. (79.2 ft. per mile).

Pier IV the west pier stands on the edge of the river bank which is 530 feet distant from the base of the bluff, the intermediate ground being a piece of bottom land of about the elevation of high water, but which is composed so largely of the tough soil known as gumbo, that the action of the river upon it is very slow. The only protection required was a quantity of riprap revetment at Pier IV and extending a moderate distance above and below; this riprap must be carefully watched and may require extension sooner or later.

The only protection on the east side is the dike, which was the first work done in the construction of the bridge, and which has acted admirably.

All the levels taken during the construction of the bridge were tied to the benches established by the Missouri River Commission and referred to a tide water datum, this datum being 412.71 feet below the St. Louis City Directrix.

#### III.

#### SUBSTRUCTURE.

The substructure comprises the four granite piers which support the channel span and sixteen small cylindrical piers which support the towers which carry the deck spans. The principal piers are numbered from I to IV, Pier I beginning at the east end. These four piers are built on pneumatic caissons of the following dimensions:—

Pier I, 53 feet long, 25 feet wide and 18 feet high.
Pier II, 55 " " 27 " " " 18 " "
Pier III, 55 " " 27 " " 18 " 18 " "

Pier IV, 53 " " 25 " " " 30 " "

All caissons were built with a side batter of one in twenty-four.

The foundations were put in by the company's own men under the direction of the Resident Engineer. The masonry was built by contract by the firm of Drake & Stratton.

The caisson for Pier I was built in a pit excavated on the dry sand bar on the east side of the river. The caissons for Piers II and III were built in position on pile false work and lowered with screws to the bottom of the river. The caisson for Pier IV was built in a pit excavated in the shore close to the river.

The pneumatic machinery was purchased from the Missouri Valley & Blair Railway and Bridge Co., and was the same machinery that had been used in sinking the foundations of the bridge at Blair crossing. The machinery was first set up on the east side of the river near the site of Pier I, Subsequently all of the machinery was transferred to the steamer John Bertram, which was purchased from the Sioux City & Pacific R. R., having been built to be used as a car transfer boat at Blair Crossing. This steamer arrived at Rulo May 28, 1886, and when equipped with the full outfit of pneumatic machinery, formed an admirable tool.

The caisson for Pier I was built of pine with an iron cutting edge and planked with two thicknesses of pine plank. The other caissons were built in the same way, except that the iron cutting edges were made heavier and the large sill timbers were of oak.

The caisson for Pier I is surmounted by 53 feet of crib work built in three sections and stepped down to 46 feet by 18 feet at the top of the upper section. The caissons for Piers II and III are surmounted by 42 feet of crib work built with the same batter as the caissons, but with the corners cut off so as to make the horizontal section that of an irregular octagon, the crib being sheathed with oak plank and the corners plated with 3-8 inch iron.

The caisson for Pier IV is surmounted by 50 feet of crib work 48 feet by 20 feet, the sides being plumb.

Both caissons and crib work were filled with Portland cement concrete.

The excavation of the pit for Pier I was begun December 3rd, 1885, the framing of the caisson December 14th and the setting up of the cutting edge January  $\tau_5$ , 1886.

The caisson was finished February 13th and the concrete filling was begun February 18th. Air pressure was put on February 20th and on March 19th the caisson reached the clay at elevation 792.1. After sinking about four feet into the clay a test pit was begun March 30th and sunk 19.3 ft. through the first clay into the gravel which separated the upper clay from the lower clay and which at the site of this pier was only 3.4 feet thick. No increase of air pressure was required during the sinking of this test pit until the gravel was reached, and then it became necessary to increase the pressure at once to the full amount corresponding to the actual depth. This test pit showed the upper clay to be a perfectly homogeneous layer on which it was considered safe to found Piers I and IV, while it was thought best to sink Piers II and III to the lower clay. The test pit having served its purpose the excavation was filled up and the sinking of the caisson was continued till April 5th when an elevation of 785.88 was reached. The sealing of the working chamber was begun on the

following day and completed on the 9th of April, thus finishing the first foundation.

No further work was done on the foundation till after the high water season, when the first foundation taken in hand was Pier IV.

The excavation of the pit for Pier IV was begun July 15th, 1886; erection of the cutting edge on July 23rd and the caisson was completed and lowered on the ground August 17th. Concreting was begun August 19th and air put on this caisson August 22nd. The caisson reached the upper clay at elevation 799.2 September 9th. The clay, though identical in character with that found at Pier I, showed signs of having been disturbed. A test pit was sunk in this clay and while it was being sunk a horizontal crack was observed on which the upper portion moved over the lower portion about 11/2 inches in a south easterly direction. It became perfectly evident that it was necessary to sink to lower clay, which was an expensive process. On the 19th of October a leak was discovered in the well leading to the air lock, and further trouble was experienced in the same way three days later, this defect being clearly due to bad workmanship. The caisson finally reached the lower clay on the 29th of October and on the 5th of November sinking was stopped at elevation 765.09 and sealing begun. The sealing was completed on the 8th of November and air let off on the following day.

No masonry was laid on either Pier I or IV until the completion of the foundation.

The first work done on Pier III was to drive a pile break water above the pier, which was begun September 11th, 1886, and this was immediately followed by the construction of the falsework. The erection of the caisson was begun October 9th and finished November 9th. Shallow water was obtained by sinking brush below the pile protection and thus forming a sand bar at the pier

site. Meanwhile, a winter bridge had been built by the Operating Department about one quarter of a mile below the bridge line, and this bridge caused an ice pack which extended above the bridge line and caused very serious trouble. On the 24th of November the caisson was lowered until it floated on the water. Concreting was begun on the following day, but the trouble with ice prevented rapid progress. On the 28th of November all access to the pier was cut off by ice and a gang of thirty men were imprisoned there till the following morning. These difficulties were over on December 1st and the construction of the crib was begun, on the 4th of December air pressure was put on. The crib was finished January 9, 1887, its concrete filling January 11th, and masonry was begun January 14th. On the 6th of January the caisson had reached the first clay; on February 10th it had passed through the first clay and reached the gravel. The second clay was reached at elevation 766.9 on March 12th. From the 1st of March to March 10th it became necessary to abandon the foundation while the ice went out. On March 18th the foundation was again abandoned just as it was ready for sealing, and during the high water of the next two weeks, the pier was entirely submerged. On the 12th of April air pressure was again put on and it was found that the caisson had settled 1.14 ft. into the clay. In order to fit the courses of masonry it became necessary to sink 1.07 ft. further to elevation 763.53. On the 19th of April the sealing of the working chamber began and the foundation was finished on

The first work done on Pier II was on the 5th of January 1887, when the driving of piles for the staging was begun. The erection of the cutting edge was begun on the 14th and the caison was finished on February 5th; it was lowered on the 10th of February, landed on the 12th and air pressure put on on the 15th, the machinery on the Bertram supplying air for both Piers II and III and

the sinking being continued in this way till the 23rd of February. Air pressure was again put on on the 18th of March; the crib and concrete filling were finished on the 8th of April, and the upper clay was reached on the 10th of April. The pier was abandoned April 12th so as to leave the steamer free to work on Pier III; work was resumed April 26th, and was then continued without interruption; the second clay was reached June 4th, the sealing of the working chamber began on the 10th, and this, the last foundation of the Rulo Bridge, was completed June 18th.

The sinking through clay was greatly facilitated by the use of a special air lock with an elevator arrangement attached by which the air pressure in the caisson was made to lift a bucket of clay to a lock above the masonry, which lock was worked entirely from the outside and when opened the bucket was dumped by the outside attendant. This special lock, known among the engineers as a "clay hoist" and among the workmen as a "go-devil" is shown in detail on Plate 10; the movement is precisely the same as that commonly used in hydraulic elevators, except that instead of water pressure, the air from the caisson was made available for power.

The full details of the four piers are given on Plates 4, 5, 6 and 7, and of the caissons on Plate 8. The rate of progress is shown graphically on Plate 9, Full records of the progress and details of sinking these foundations were kept and are given in Appendix D. The detail cost is given in Appendix E.

The cost of the four foundations is shown in detail in the table below.

This cost includes all concrete and other material below the masonry.

In this statement the item of freight charges is simply what is known as "company's freight" being freight on the lines of the C. B. & Q. R. R. system.

### THE RULO BRIDGE.

	Cost, exclud- ing Freight Charges.	Preight Charges.		lost, including	Freight Charg	:5.		Cost, exclud- ing Freight Charges.	Freight Charges.		Cost, including	Freight Char	ges.
DUNDATION I:							FOUNDATION III:				1		
CAISSON— Materials Labor	\$3,879.25 1,928.17	\$ 569-53	\$ 4,448.78 1,928.17	\$ 6,376.95			CAISSON— Materials Labor	\$3,265,21 4,077.20	\$ 983.55	\$4,248.76 4,077.20	\$ 8,325.96		
CONCRETE FILLING— Materials	4,044.22 1,536.99	1,153.02	5,197,24 1,536,99	6,734.23			FALSE WORK— Materials Labor	785.97 1,249.73	119.91	905.88 1,249.73	2,155.61		
CRIB— Materials	3,935.11 1,264.97	1,121.05	5,056,16 1,264,97	6,321,13	\$13,111.18		CONCRETE FILLING — Materials Labor	4,036.01 3,317.58	911.76	4,947-77 3,317,58	8,265.35		
CONCRETE FILLING— Materials Labor	6,889.20 2,306.13	1,372.85	8,262.05 2,306.13	10,568.18			CRIB— Materials Labor	3,286,68 2,664.74	994.89	4,281.57 2.664.74	6,946.31	\$18,746.92	
CUTTING EDGE, AIR-LOCK, SHAFTS, ETC	2,124.96	303-44	2,428.40	-	2,428.40		CONCRETE FILLING— Materials Labor	7,253.66 3,884.73	1.274.60	8.528.26 3,884.73	12,412.99		
Materials	1,184.76 9,440.52	297.64	1,482,40 9,440,52		10,922.92		CUTTING EDGE, AIR-LOCK, SHAFS, ETC	4,256.63	315.89	4,572.52		19,359.30 4,572.52	
ERECTION AND REMOVAL OF MACHINERY	1.999.85	4,817.53	1,999 85	-	1,999.85	\$45,351.66	SINKING— Materials Labor	2,223,83 19,697.44	537-71	2,761.54 19,697.44		22 140 -0	
UNDATION IE						- '	ERECTION AND REMOVAL OF MACHINERY					22,458.98	
CAISSON— Materials Labor	3,314.51 3,079.51	1,053.24	4.367.75 3,079.51	7,447.26			FOUNDATION IV:	520.59	5,138.31	520.59	_	520,59	\$65,0
FALSE WORK— Materials Labor	1,051.86 981.68	291.12	1,342.98 981.68	2,324.66			CAISSON— Materials Labor	3,980.38 2,551.45	806.05 —	4,786.43 2,551.45	7,337.88		
CONCRETE FILLING— Materials Labor	4,182.06 3,169.65	1,101.70	5,283.76 3,169.65	8,453.41			CONCRETE FILLING— Materials Labor	4,092,87 1,917.96	550.00	4,642.87 1,917.96	6,560,83		
CRIB— Materials	2,648.81 2.349.19	860.87 —	3,509,68 2,349.19	5,858.87	18.225.33		CRIB— Materials Labor	2,586-37 2,134 96	497.66	3,084.03 2,134.96	5,218,99	13,898.71	
CONCRETE FILLING— Materials Labor	6,371,15 3,896.89	1,600.74	7,971.89 3,896.89	11,868.78			CONCRETE FILLING— Materials Labor	7,088.43 1,986.97	1,329.03	8,417,46 1,986,97	10,404.43		
CUTTING EDGE, AIR-LOCK, SHAFTS, ETC	3,850.65	344-34	4.194.99	_	4,194.99		CUTTING EDGE, AIR-LOCK, SHAFTS, ETC	3,155.20	337-53	3,492.73	-	15,623.42 3,492.73	
SINKING— Materials Labor	2,001.54 15,827.67	557.63	2,559.17 15,827.67		18,386,84		SINKING— Materials Labor	1,733.42 15,047.99	373,58	2,107.00 15,047.99	_	17,154.99	
ERECTION AND REMOVAL OF MACHINERY	\$49.86 \$53,275.03	- \$5,809.64	549.86	-	549.86	\$70.084.67	ERECTION AND REMOVAL OF MACHINERY	478.26		478,26	_	478.26	
						\$59,084.67		46,754.26	3.893.85				\$50,64
							GRAND TOTAL COST OF FOUR MAIN FOUNDATIONS	\$201,083.42	\$19,650,33				\$220,74

It will be observed that the cost of the foundation of Pier III was \$6,574 more than that of Pier II although the quantities of material are almost identical in the two piers, the extra cost being due to the interruption and other difficulties from ice and the incidental troubles connected therewith.

The contract for the masonry was let on July 23rd, 1886 to Geo. S. Field & Co. and transferred by them on the 2nd of August with my consent to the firm of Drake & Stratton. The dimension work is of granite quarried near St. Cloud, Minn; the backing is of Anamosa limestone from Stone City, Iowa.

The specifications for the masonry are given in Appendix C.

The first stone laid was on Pier IV, the masonry of this pier being begun November 27th, 1886 and finished March 25th, 1887. The masonry of Pier III was begun January 14th, 1887 and completed May 19th, 1887. The masonry for Pier II was begun April 12th, 1887, and finished August 10th, 1887. The masonry for Pier I was begun April 19th, 1887, and completed August 9th, 1887.

The cost of masonry is shown in detail in the following table:

		PIEI	₹ 1.			PIE	R II,			PIE	R 111.			PIE	R IV.	
Estimate—  Masonry laid @ \$27.40 per cubic yard	cu.yds. 561.80		_	Fre our or	cu.yds.				cu. yds.				ou.yds.			
Labor—Replacing Course	501.00		_	\$15,393.32	128,59	_	-	\$35,233.66	1.317.00	-	_	\$36,085,80	814.40	-	_	\$22,314.5
Cutting and Drilling for Cramps	= 1		_				-	9.10	-	-	-	5-30	-	_	-	-
130 lbs. 1/4 in, Round Iron for Cramps					_	_	=	32.40	-	1	7	-	-	_	Stand	-
	-	-		-	-	-	-	3.90	****	-	=		-	-	-	-
Freight Charges on Stone	-	-	\$655.09	-	-	reed	\$1,599.56	-	-	-	\$1,515.69	-	_	-	\$993.62	
Freight Charges on Sand	print.	-	126.23	781.32	-	-	289.93	1,889.49		-	295.91	1.811.60	-	-	182.95	1,176.5
	bbls.	-	-	-	bbls.	-	-	-	bbls.	-	-	-	bbls.	-	-	
Eagle Cement @ \$2.55 per bbl ,	107	\$273.75	-	-	-	-	-	-	DDIS.	-	-	-	-	-	-	-
Freight Charges	-	100.57	374.32	-	-	-	-	-	-	-	-	-	-	-	-	-
Alsen Cement @ \$2.70 per bbl	2.4	64.80	-	-	202	\$545.40	-	-	428	\$1,155.60	-	_	389	\$1,050.30	-	-
Freight Charges , ,	-	10.34	75.14	-	-	100.43	645.83	-	-	254.76	1,410.36	-	-	191.77	1,242.07	9 —
Alsen Cement @ \$2.80 per bbl	26	72.80	-	-	187	523.60	_	-	_	_	_	=	-	-	-	-
Freight Charges	=	10.92	- 83.72	-	9-2	92.97	616.57	-	-	-	-	-	-	-	-	-
Alsen Cement @ \$3.35 per bbl	69	224.25	-	t-r	5	16.25	-		-	_	_	_	-	_	-	_
Freight Charges	-	9.90	234.15	-	-	81.	16.43	-	-	-	-	-	-		-	·
Dyckerhoft Cement @ \$2.72 per bbl	60	163.20	-		233	633.76	-	-	220	598.40	_	_	52	141.44	-	-
Freight Charges	-	29.77	192.97	960.30	-	115.85	749.61	2,028.44	-	130.96	729.36	-	-	25.63	167.07	1,409.1
Total , . ,	-	-	-	\$17,134.94	-	-	-	\$39,196-99	-	-	-	\$40,043.42	-,	-	-	\$24,900.
Cost per Yard of Masonry	-	-	-	30,50	-	_	-	30.48	-	-	_	30.40	-	-	-	30.
everage Cost per Yard of Masonry	No.	_	_		_	_	_	_	_	_	0000	_	<u>.</u>	_	_	30

The total cost of the four piers including the foundations and masonry is given in the following table:

	Cost,	excluding Fre	ight.	F	reight Charges		Cost	t, including Fre	ight.	Gross Volume.	Cost per Cubic Ft.	Cu. Ft. Sunk (Area of base × feet sunk.)	Cost per Cubic Ft.	Vertical Ft. Sunk below Standard Low Water.	Cost per Vertical Foot
FOUNDATION I:  Caisson and Filling.  Crib and Filling.  Sinking Caisson  Erection and Removal of Machinery	\$13,513.59 14,395.41 10,625.28 } 1,999.85 }	  \$40,534.13	=	\$2,025.99 2,493.90 297.64	- - - \$4,817.53	111	\$15 539 58 16,889.31 12,922.77	- - - \$45,351.66	=	Cubic Feet. 23,093,16 53,568,12	Cents. 67.2 31.5	760.02	Cents.	57-36	\$220.10
FOUNDATION II:  Caisson and Filing Crib and Filing Sinking Caisson Erection and Removal of Machinery.	19,629.92 15,265.04 17,829.21 } 549.86 }	53,275.03	Ξ	2,790.40 2,461.61 557.63		=	22,420,32 17,727,05 18,936.70	   59.084.67	=	25,930.31 45,764.09 —	86.4 38.7 —	115,488.45	15.9	77 -77	236.34
FOUNDATION III:  Caisson and Filling Crib and Filling Sinking Caisson Erection and Removal of Machinery .	20,988.33 17,089.81 21,921.27 { 520.59 }	- - - 60,520.00	Ξ	2,331.11 2,269.49 537-71		Ξ	23,319-44 19,359.30 22,979.57	E 65,658.31	=	25,930,31 46,015,00	89.9 42.1	118,369.35	19.0	79.71	281.76
FOUNDATION 1V:  Caisson and Filling	15,697.86 13,796.73 16,781.41 478.26 §		1.1	1,693.58 1,826.69 373.58	3,893.85	=	17,391.44 15,623.42 17,633.25	50,648.11	= -	27,368.21 49,563.36	63.5	103,548.75	16.6	78.15	219.51
COST OF FOUNDATIONS	-		\$201,083.42	-	-	\$19.659.33	-	-	\$220,742.75						
MASONRY PIER I III III III III III III III III II	=	16,192.12 36,998.07 37,845.10 23,506.30		Ē	942.82 2,198.92 2,197.32 1,393.97	6,733.03	=	17,134.94 39,196.99 40,042.42 24,900.27	121,274,62						
TOTAL COST OF FOUR PIERS,	-	-	\$315,625.01	-	-	\$26,392,36			\$342,017.37						

The towers which carry the approach spans are supported on brick piers, the plans of which are given on Plate 12. An excavation was made at the site of each pier in which nine piles were driven; a further excavation was then made around the piles and filled with concrete. In this concrete was buried an annular washer from which six anchor rods extended upwards. The pier was then built up with hard brick laid in Portland cement mortar around the rods and surmounted with a wrought iron cap plate. Another annular washer was then placed on this cap plate and the rods screwed up till an initial strain of five tons was obtained in each rod: the cap plate, which was formed of a plate and a circular channel iron was then filled with Portland cement concrete; a second cap plate was then put on top and the whole riveted up. The design was based on the principle that the tension in the rods would always keep the brick work under strain and so prevent its jarring loose. These piers were built by the day by the company's own men, the total cost of the 16 piers being \$33,264,66 of which \$1,479.55 was for freight.

The amount of masonry and concrete in the entire bridge is as follows:

			MASONRY, Controls	Filt K On a lines	CONCRETE. Cubic Yards	TOTAL.
Pier I	 ,		561 8		2,109.8	2,671,6
Pier II		,	1,285.9		1,986,1	3,372 0
Pier III			1351		1 949.0	3,266 a
Pier IV			814.4		2,267.0	3,081.4
16 Small Piers .				263.8	266.7	530.5
Total			3.0% (	263,8	8,578,6	12.821.5

The total cost of the substructure was then as follows:

Four Main Pers .			,	\$342,017.37
Sixteen Small Piers				23,264.66
				4 4 0

The east end of the east deck span rests on a timber pier around which the embankment has now been filled; the west end of the west approach span rests upon a concrete block resting on the embankment and allowed to settle with the embankment, the end of the span being raised as settlement occurs. Both the timber pier and the concrete block have been treated in the accounts as parts of the approaches, though this distribution is not strictly correct.

### IV. SUPERSTRUCTURE.

The superstructure consists of three through spans and six deck spans, three at each end.

Each through span is 375 feet long between centers of end pins, fifty feet deep and twenty two feet between centers of trusses. Expansion is provided at the west end of every span, that is at the upper end, the bridge being

Each deck span is 125 feet long between centers of end pins, 17 feet 6 inches deep, the trusses placed 12 feet between centers. The spans are separated by iron towers 25 feet long, thus making each set of deck spans with intermediate towers a continuous structure 425 feet long, divided into 17 equal panels of 25 feet each. The trusses are fastened rigidly to the posts which form the towers; expansion is provided at each end of the 425 feet, the expansion of the central span being taken out by the spring of the towers.

Proposals were invited from a number of prominent bridge builders and on the 2d of August 1886 the contract for the superstructure was closed with the Edgemoor Iron Co., by whom the entire work was manufactured. With my approval the Edgemoor Iron Co. sub-let the erection to the firm of Baird Bros., and it was done under the immediate direction of Mr. Andrew Baird.

The through spans are of the double system Whipple type, the trusses being divided into 15 panels of 25 feet each. The top chord, end posts, eye bars, floor beams, rods, bolsters, rollers, rail-bearing plates and pins are of steel. All other parts are of wrought iron except the heavy wall plates rest. ing on the masonry, washers and ornamental work, which are of cast iron. The details of these spans are given on Plates 14, 15, 16, 17 and 18.

The deck spans are of the single system Whipple type. The pins, rollers, rail-bearing plates and eye bars are of steel. The other portions are of wrought iron except the heavy wall pedestals, which are of cast iron. The details of these spans are given in Plates to and 20.

The trusses of the long spans were proportioned to carry a uniform moving load of 3000 lbs. per lineal foot of bridge, the effect of a moving load in excess of that due to a uniform load of equal intensity being estimated on the basis of 5000 lbs. per foot.

The floor system is proportioned for a total load of 6000 lbs. per lineal

foot of track. The top lateral system is proportioned to resist a wind pressure of 300 lbs. per lineal foot and the bottom lateral system a wind pressure of 500 lbs. per lineal foot. The computed strains are given on Plate 22.

The deck spans are proportioned for a total load of 5000 lbs. per foot, all treated as moving load.

The steel compression members in top chords and end posts are made as nearly as possible of symmetrical section, the metal in the top cover plate being practically the same as in the two balance plates and the lacing below. The compression strains on these members was limited to 15,000 lbs. per square inch of net section, the net section being obtained by deducting from the gross section the amount by which the cover plate exceeds the balance plates. The tensile strain in the bottom chord was limited to 14,000 lbs. per square inch and somewhat less in the web members.

In the approach spans the tensile strain on steel was limited to 13,000 lbs, per square inch, the largest strain being in the center panel of the bottom

The weights of iron and steel in the through spans are as follows:

	a 30	Spans.	Av rage per Span.		
Steel	Ibs	15s. 1,554,168	lbs	518,056	
Wrought Iron in Trusses	860,127		386,709		
Wrought Iron in Floor	453,867		61 374		
Total Wrought Iron		3+3 4/14		447,998	
Cast Iron		60 0 1/1		20,019	
Total		2,958,218		43.03	

The weights of iron and steel in the deck spans are as follows

	~ x S	pans.	v. rage	Str
	lbs	Its	hs	lbs 28,396
Wrought Iron in Trusses	343 513		57-319	
Wrought Iron in Floor	1 60		46,378	
Total Wrought Iron		622,179		103,697
Cast Iron		7 **		1,182
Гоtаl		799,652		133.775
Four Towers		313441		
Total		1,013,093		

The specifications under which the superstructure  $% \left( x\right) =\left( x\right) =0$  was manufactured are given in Appendix F.

The steel was all open hearth steel, the total number of melts used being 161, made by the following parties:

Cambria Iron and Steel Co. 82 melts
Carnegie, Phipps & Co., Limited 52 "
Pennsylvania Steel Co. 5 "
Pittsburgh Steel Casting Co. 22 "

The work was manufactured by the Edgemoor Iron Co. at its works near Wilmington, Dela.

The first set of eye bars tested did not meet the requirements of the specifications, a considerable number of them breaking in the head; it seemed probable, however, that the breakage was not due to any defect in the bars, but to the fact that the machine was not strong enough to break them and that its own failings caused irregular strains. I therefore thought best to open the question anew and make the rejection of the bars depend on a second set of tests to be made in the large testing machine at Athens, Penna; the results of these tests are given in Appendix G and on these tests the eye bars were accepted.

The trusses were erected on pile false work, a large traveler taking the place of upper false work. Thedates at which the several parts were erected is shown in the following table:

East Approach	Spans	,	,		First Iron Placed. Sept. 4, 1887.	Span Swung Sept. 14, 1887
Span I-II				,	Sept. 18, 1887.	Sept. 24, 1887
Span II-III .					Aug. 14, 1887.	Aug 18, 1887
Span III IV .			4	,	May 21, 1887.	May 27, 1887
West Approach	Spans				April 13, 1887.	Tuly 20, 1881

The last span would have been swung three days earlier but was delayed waiting for material.

The timber floor was placed on the superstructure by the company's own men, working under the direction of the Resident Engineer. The painting was also done by the company's own men working under the direction of the Resident Engineer.

The total cost of the superstructure is given in the following table:-

THROUGH SPANS			
ron, Steel and Ornamental Work reight Charges on same from Chicago Labor,—Erection Miscellaneous Material: Cement, Iron Berings, Sal Ammonia, Sulphur DECK SEANS,	\$103,827.91 4,001 34	\$107,829.25 29,730.00 57.75	\$137 617 ∝
ron and Steel	33.854.76 1,442.40	25.24.16	
.abor—Erection Filing Castings Jacking up End of Spans Boxing Bars Switching Charges Miscellaneous Materia: Iron Borings, Sal Ammonia &c,	=	35.397.16 7.499.00 13.85 37.81 18.00 7.75 55.34	
FLOOR.			41,918 41
Material	Ξ	7.771.23 .92.48 .502.12	11,495 8
PAINTING.			
Material	3	1.340.95	5,448.81
Total Superstructure			\$196,480.06

V.

#### APPROACHES.

The east approach to the Rulo Bridge is 2.88 miles long from the con nection with the track of the St. Joseph & Nebraska R. R. to the east end of the iron work. Of this approach the 2000 feet next to the bridge was built originally as a timber trestle and subsequently filled in with material hauled from the great cut on the west approach. The remainder of the approach was built as an earth embankment, the material being generally borrowed from the sides, though a portion of the higher embankment was taken

with a steam shovel from a pit in the bottom land. The total amount of earth in this Approach is as follows:—

The west approach is 3.64 miles long from west end of the iron work to the connection with the Atchison & Nebraska track in the Nemaha bottom. There are on this Approach three cast iron pipe culverts and four small trestes having an aggregate length of 495 feet. All the remainder is earth work the total quantity being as follows:

Earth	excavation	hauled to east side	326,970	cubic	yards
44	f a	used in embankment	240,173	44	6.4
	**	wasted .	32,358	6.6	0
E <sub>b</sub>		borrowed	44,340	44	66
Rock	•		2,700	6.	
	Total		646,541	cubic	yards

These quantities include an extra width of grading for a distance of 1200 feet at the Rulo station grounds.

The Atchison connection is 1.36 miles long leading southward from the west approach, with which it connects 760 feet from the west end of the iron work. There is a trestle 680 feet long in the town of Rulo, and a pile trestle 48 feet long near the connection with the old track, the remainder of the line being earthwork. The amount of material handled on this line was 42,718 cu bic yards.

The alignment and gradients on the Approaches are shown on Plate 2.

The contract for the earthwork of the Approaches was let May 19, 1886, to S. Dwight Eaton, of Burlington, Iowa. This contract covered both the East and West Approaches and the Atchison connection; it was not signed till lune 26, 1886.

Grading was begun on the Atchison connection June 18, 1886, on the West Approach June 22d and on the East Approach July 13, 1886. The line from the east end of the great cut to the connection with the Atchison & Nebraska track was not built till after the bridge was opened, the line to the west being made by way of the Atchison Connection, trains running backwards between Rulo Station and Rulo Y, although the bridge was opened for traffic on the 2d of October.

No earth was taken across the river from the great cut till November 23, 1887. More than seven weeks of the best season of the year being lost through unnecessary delays; from that time forward the work was handled with rather more efficiency and the entire West Approach was completed and ready for the track on June 3d, 1889.

The great cut has a maximum depth of 82 feet. It is excavated through material of a mixed character, it being generally a sandy day but containing some masses of hard blue clay and pockets of sand, these pockets of sand were generally filled with water, which water ran out when the excavation was made and did not reappear. The bottom of the cut is formed everywhere of hard clay, excepting near the summit at the west end. The cut was laid out with a 50 foot base with slopes of one horizontal to two vertical. In some instances these slopes were slightly increased, and some of the material was left in the base so that the finished width was generally not more than

40 feet. The cut was thoroughly ditched, left of sufficient width to be kept clean permanently, and the track was thoroughly ballasted. The character of the material, however, in this cut is such that it will require constant atten tion for a number of years in keeping the ditches clean, so as to avoid saturating the material in the sides of the cut by standing water; I regret to say that this has not been properly attended to since the work was completed.

A line nine tenths of a mile long was built connecting the East Approach with the dike above the bridge, the only object being to make a convenient line of access in case any repairs should be needed after the old tracks on the east side should be taken up; this line has at the east end a grade of three per cent. 158.4 ft. per mile.

# VI. PROTECTION WORK.

The principal protection work is the dike on the east side. The position and plans of this dike are shown on Plate 23. The foundation of the dike was made of a woven willow mattress 125 feet wide, extending 100 feet

inside and 25 feet outside of the center line of the track laid on the dike. On this as a foundation was built an embankment of brush and rock, which was carried to an elevation of 856 and on which a track was laid. This dike was built by the company's own forces in the winter of 1884 85.

The dike was subsequently extended 700 feet down stream to Pier I by a permeable screen made by driving a series of piles through a woven mattress 50 feet wide and subsequently putting another mattress on the outside of the piles the lower edge resting on the first mattress and the upper edges wired to the piles. The effect of this screen was to allow the river to permeate through the screen so that there would be a current on both sides of the screen and thus entirely prevent the formation of an eddy at the lower end of the dike.

The dike worked perfectly, and during the high water season of 1885, a deposit was formed below it nearly as high as the top of the dike. A good growth of willows now covers the ground between the dike and the bridge line.

There was used in the construction of the dike 3571 cords of brush, 8712 tons of riprap stone, 2273 teet of piles and 4223 lbs. of wire.

The only protection work done on the west side was to riprap the shore in the neighborhood of Pier IV; for this 2530 tons of stone were

VII.

The cost of the bridge and approaches is given in the following table:

	, introduce	of Ereight "larges		Preight Charges		3	st in's ding Fre gi	1
Protection East Shore Protection West Shore Total Protection	7 4 600 V	\$1045¥3 ↔	\$4,028.63 13( \s		\$4.,85.45	\$18,523,65		\$ 21,528.9
Coundation Pier II Coundation Pier III Coundation Pier III Coundation Pier IV Total Foundation Pier IV	40,534.13 53,275 03 60,520.00 46,754.36	H.2063 42	5,50,64 5,50,64 5,3831 3,593,85	\$19,659.33		45,351 66 5 ,054 t 132 33 3 50, 48 1	£200,042.75	
Masonry Pier I Masonry Pier III Masonry Pier III Masonry Pier III Total Masonry	36,998,07 37,845,10 37,804,30	14,541.59	942,82 2,198,92 2,197,32 1,393,97	6,733.03		17,134,94 39,195,99 40,04141 24,900,17	111,274/2	
Approach Piers		21,786,11		1,478 55			23,264.66	
Total Substructure		337,411.12			27,870.91			365,2824
hrough Spans beck Span roidge Floor atating Total Superstructure	133,615,66 40.4**6.1 10.5**3 35 5.448.82	190,113,84	4.001 34 1,442 40 927 48		6,366 32	137,617.00 11 ,15 41 11,49 <sub>2</sub> 83 5,448 83		196,480.
ast Approach, Grading and Masonry  ast Approach, Trestle yest Approach, Grading and Masonry  Yest Approach, Trestle yest Approach, Trestle g and Masonry  tethson Connection Trestle  remanent Track  Total Approaches	74,813,56 18,907,08 151,481,64 31,2 % 33 8,63,2 12 6,906,34 72,705,99	336,975,16	* 881 20 3,242.79 6,528 08 403,68 70.67 350.11 4,319.91		17,796.44	77,694.76 23,149.87 158,005,72 3,932.11 8,702.79 7,356.45 77,025.90		354,771.
Service Tracks, Fools and Machinery Juildings	14,314.16 12,327.42 3,500,51	30,442.09	680.58 1,467.11 197.69		2,345.38	14.994.74 13.794.53 3,998 °o	1	32,787.
Engineering Salaries	45,400,64 4,134.03	49,534.67				45,400.64 4,134.03		49,534
Total cost		\$961,020.32			\$ 59,364.43			\$1,020,384

The item of freight includes freight only over the C. B. & Q System. In comparing the cost of this bridge with that of other structures the cost without freight forms the most correct basis for comparison

This table may be condensed into the following:

	Cost exclusive of Freight Charges	Freight Charges.	Preight Charges
Substructure	\$337.411 12	\$27.870 91	\$365,282.03
Superstructure	190,113.84	6,366.22	196,480.06
Lita Budge rope	576747	34.237 13	561,762 09
Protection Works	16,543 44	4.985 48	21,528,92
Approaches	336,975.16	17,796.44	354.771.60
Service Tracks, Tools and Buildings	30,442 09	2,345 58	32,787.47
Engineering and Expenses	49 534-67		49,534.67
Tut Cost	Syn, ora 37	\$59,364.43	\$1,020,384.75

This is the total cost of the Rulo bridge and approaches as built under my charge; the following additional items have, however, been charged to the cost of the bridge:

Land	Dama	ıge	S							\$46,721.35
Watcl	hing					,		,		5,296.91
Prelin	ninary	E	хр	ens	es					340.25
Rulo	Yard							,	,	98 34
										\$62.456.85

Which makes the total cost \$1,072,841.60. Against this the construction cost is really entitled to a considerable credit for the amount of abandoned line on each side of the river. The item of watching covers the 25 months from October 1, 1887 to November 1, 1889, during which whole period the bridge was in use.

### APPENDIX A.

### LIST OF ENGINEERS. CONTRACTORS AND EMPLOYEES.

### CONTRACTORS. ENGINEERS AND COMPANY'S EMPLOYEES. NATURE OF WORK. TIME OF SERVICE Drake & Stratton . . . . . . . . . . . . . . . . . Masonry. Geo. S. Morison, Chief Engineer. lames Doig, Supt. at Rulo. Benjamin L. Crosby, Resident Engineer . Sept. 13, 1883, to Dec. 31, 1889 Edwin Duryea, Jr., Assistant Engineer . Mar 22, 1886 ° Nov. 21, 1886 Mark A. Waldo, Assistant Engineer . May 29, 1886, " June 76, 1887 Edgemoor from Co. . . . . . . . . . . . Superstructure. Baird Bros., Sub-Contractors for Erection Mark A. Waldo, Assistant Engineer May 29, 1886, " June 16, 1607" W. S. Macdonald, Assistant Engineer Aug. 9, 1886, " Feb. 25, 1889 A J. Himes, Assistant Engineer July 7, 1888, " Oct. 31, 1889 W. R. Johnson, Rodman Nov 17, 1885, " Oct. 31, 1889 J. M. Richardson, Clerk July 8, 1886, " Oct. 31, 1889 R. F. Thayer, Timekeeper June 23, 1886, " Aug. 13, 1887 S. Dwight Eaton . . . . . . . . . . . . Grading Approaches. J. S. Wattles . . . . . . . . . . . . . . . . . . East Approach Trestle. E. P. Butts, Inspector of Stone at Quarries . . . . Sept 1, 1886. ' July 15, 1887 John Naegeley, Inspector of Superstructure . . . Sept. 20, 1886, "Oct. 6, 1887 Paul Willis, Assistant Inspector of Superstructure . Dec 15, 1886, "July 13, 1887 J. RUCK, Profession of Carpenters. June 10, 1886. Apr. 7, 1887. Birton Reed, Inspector of Masonry Nov 10, 1886, 1887. Feb. 45, 1887. Charles Stream, Inspector of Masonry May 4, 1887. July 1887.

John Newman, Foreman of Laborers . . . . . Nov 28, 1884, " Feb. 25, 1887

### APPENDIX B.

#### ACT OF JUNE 18, 1884, AUTHORIZING CONSTRUCTION OF RULO BRIDGE AND CONTRACT WITH WAR DEPARTMENT.

An act to authorize the construction of a bridge across the Missouri River at some accessible point within ten miles north and ten miles south of the town of Rule, in the county of Richardson, in the State of Nabracks

BE IT ENACTED BY THE SENATE AND HOUSE OF REPRESENTATIVES OF THE UNITED STATES OF AMERICA IN CONGRESS ASSEMBLED:

That the Atcheson and Nebruska Railway Company, an incorporation organized under the laws of the State of Nebruska, is bereby authorized to construct and maintain a bridge across the Missouri River at such a point as may be bereafter selected by said corporation within ten miles north and ten miles south of the town of Rulo, in the county of Richardson, in the State of Nebruska, as shall best promote the public convenence and welfare and the necessates of business and commerce, and also to construct accessory works to secure the best practicable channel-way for ravigation and confine the flow of the water to a permanent channel at such pout, and also to lay on and over said bridge a rulway track for the more perfect connection of any mirruhast that are or shall be constructed to suit river at or opposite said point; and said corporation may construct and maintain ways for wagons, carriages, and for foot-passengers, charging, and receiving reasonable to the therefor as may be approved from time to time by the Secretary of Was

SEC. 2. That such bridge shall be constructed and built without interference with the security and convenience of naregition of such river beyond what is necessary to carry into effect the rights and privileges hereby granted, and in order to secure that object the said company or corporation shall submit to the Secretary of war, for his examination and approval, a design and drawings of the bridge, and a map of the location, groung, for the space of one mile above and one mile below the proposed location, the topography of the banks of the river, the shore-time at high and low water, the direction and strength of the currents at all stages, and the soundings, accurately showing the bed of the stream, the location of any other bridges, and shall farnah such other information as may be required for a full and satisfactory understanding of the subject, and until the said plan and location of the bridge are approved by the Secretary of War the bridge shall be for the bridge.

PROVIDED, That if the said bridge shall be made, with unbroken and continuous spans, it shall have three or more channel spans, and shall not be of less elevation in any case than fifty feer above extreme high-water mark, as understood at the point of location, to the bottom chord of the bridge, nor shall the spans of said bridge be less than three hundred feet in length, and the piers of said bridge shall be parallel with the current of said ever, and the main span shall be over the main channel of the river, and not less than three hundred feet it length.

AND PROVIDED ALSO, that if any bridge built under this act shall be constructed as a draw-bridge, the same shall be constructed as a pivot-bridge, with a draw over the main channel of the river at an acessible and navigable point, and with spans of not less than one hundred and sixty feet in length in the clear on each side of the central or pivot p.er of the draw, and the next adjoining span or spans to the draw shall not be less than there hundred feet, and the head room under such span shall not be less than ten feet above high water mark.

PROVIDED ALSO, That said draw shall be opened promptly upon reasonable asgnal for the passing of boats; and said company or corporation shall maintain, at its own expense from sunset till sunnise, such lights or other signals on said bridge as the Laght-House Board shall prescribe:

PROVIDED ALSO, That all railway companies desiring to use said bridge shall have and be entitled to equal rights and privileges in the passage of the same, and in the use of the machinery and fixtures thereof, and of all the approaches therefor, and of all the approaches the return and conductions as shall be prescribed.

by the Secretary of War, upon hearing the allegations and proofs of the parties, in case they shall not agree

SRC. 3. That the Secretary of War is hereby authorized and directed, upon receiving such plan and map and other information, and upon being satisfied that a bridge built on such plan and with such accessory works and a teach locality will conform to the prescribed conditions of this act, to notify the company that he approves the same, and upon receiving such notification the said company may proceed to an erection of said bridge, conforming strengt by the approved plan and locationy and should any change be made in the plan of the bridge or said accessory works, during the progress of the work thereon such change shall be subject likewise to the approval of the Secretary of War, and in case of any hitgation arising from any obstruction or alleged obstruction to the free navigation of said river caused or alleged to be caused by said bridge, the case may be brought in the circuit court of the United States of the State of Nebraska or State of Iowa in which any portion of said obstruction or bridge may be located.

SEC, 4. That the said bridge and accessory works, when built and constructed under this act and according to the terms and limitations thereof, shall be lawful structures; and said bridge shall be recognized and known as a post-route, upon which also no higher charge shall be made for the transmission over the same of the mash, the troops, and the monitors of war of the United States than the rate per mile paul for the transportation over the railtrade or public highways leading to said bridge and said bridge shall elopy the rights and privileges of other post routes in the United States, and Congress reserves the right at any time to regulate by appropriate legalation the charges of regist and passengers over said bridge.

SEC. 5. That the United States shall have the right of way for such postal-telegraph lines across said bridge as the Government may construct or control.

SEC. 6. That Congress shall have power at any time to alter, amend, or repeal this act so as to prevent or remove all maternal and substantial obstructions to the navigation of said river by the construction of said bridge and its accessory works, and the expense of altering said bridge or removing such obstructions shall be at the expense of the owners of or persons controlling such bridge.

Received by the President, June 6, 1884.

(NOTE BY THE DEPARTMENT OF STATE. The foregoing as having been presented to the President of the United States for his approval, and not having been returned by him to the Rouse of Congress which it originated within the time prescribed by the Constitution of the United States, has become a law without his approval.)

#### CONTRACT.

WHEREAS, By an Act of Congress of June 18, 1884,—23 Stats 45—entitled, "An Act to authorize the construction of a bridge across the Missoura River at some accessible point within ten miles south of the town of Rk.lo, in the county of Kichardson, in the State of Nerbasia," twas cancel that the Atchison & Nebrasia Railway Company, an incorporation organized under the laws of the State of Nerbasia, in thereby authoraced to construct and maintain a bridge across the Missoura Kiver at a point within ten miles north and ten miles south of the town of Rulo, in the county of Richardson, in the State of Nebrasia, and to construct accessory works to secure the best practicable channel-way for navigation, and also to lay on and over saud bridge a raiway track to construct and mentain ways for wagons, carnage and foot passengers, charging and receiving reasonable foll therefor as may be approved from time to time by the Secretary of War, and

WHRREAS, It was further enacted by the Act of Congress aforessud, that the said bridge shall be constituted and bailt without interference with the security and convenience of navigation of said river beyond what is necessary to carry into effect the rights and privileges bereby granted; and in order to secure that object the said company or corporation shall submit to the Secretary of War for his examination and approval, a design and drawings of the bridge, and a map of the location, giving, for the space of one mile above and one mile below the proposed location, the topography of the banks of the river, the shore lines at lugh and low water, the direction and strength of the currents at all stages, and the soundings, accurately showing the bed of the stream, the location of any other bridge or bridges, and shall furnish such other information as may be required for a full and satisfactory understanding of the subject; and until the said plan and location of the bridge and an opposed by the Secretary of War, the bridge shall not be built, and

WHEREAS, The Secretary of War is authorized and directed by said Act of Congress, upon receiving such plan and map and other information, and upon being satisfied that a bridge built on such plan and with such accessory works and at such locality will conform to the prescribed conditions of said Act, to notify the commany that he approves the same, and

WHEREAS, The Atchison and Nebraska Railway Company in accordance with the provisions of the Act of Congress aforesaid, has submitted to the Secretary of War for examination and approval its design and drawing of the sud bridge, and a map of the location of the same, as by said Act of Congress required, and

WHEREAS, The Chief of Engeners, United States Army, has reported, "That from the best and most reliable information attunable by this office, contained in the papers herewith, it is believed that the plans of the bridge at Rulo are substantially in accordance with the requirements of the Act of Congress' aforesaid.

NOW THEREFORE, I, William C. Endicott, Secretary of War, having examined and considered the plan and location of the bridge submitted by the said Atchison and Nebraska Railway Company, as aforesaid, which said plan and map of location are hereto attached and form part of this instrument, do hereby annowe the same.

But it is understood and agreed that this approval is given upon the express conditions following:—

- That the said bridge shall be erected at the point indicated in the map of location submitted, and be constructed in accordance with the provisions of suid Act of Congress, and the plan submitted and attached as a forestal.
- That should any change be made in the plan of said bridge during the progress of construction such change shall be subject to the approval of the Secretary of War.

Witness my hand this 25th day of February, 1887.

WM, C. ENDICOTT, Secretary of War.

G. W. HOLDREGE, President

This instrument is also executed by the Atchison and Nebraska Railway Company, by its President G. W. Holdrege, thereto lawfully authorized this 12th day of February, 1887, in testimony of its acceptance of the sord-wors of the said Act of Convress, and the conditions therein immost of the said Act of Convress, and the conditions therein immost of the said Act of Convress, and the conditions therein immost of the said Act of Convress, and the conditions therein immost of the said Act of Convress, and the conditions therein immost on the said of the sai

THE ATCHISON AND NEBRASKA RAILWAY CO.

In presence of

THOS, MILLER, WM, A. HIGGINS, C. D. DORMAN.

Attest, J. C. TAYLOR, Secretary.

### APPENDIX C.

#### SPECIFICATIONS FOR MASONRY.

There will be four piers, numbered from east to west. Piers I and IV will be within the shore lines, and will contain approximately 350 and 800 cubic yards and The manonry. Piers II and II will be in the prive, and will contain approximately 1,300 cubic yards each. The manonry of Piers I and IV will be sourted on finished concrete foundations, and above the surface of the water. The manonry of Piers II and III will rest on foundations put in by the plenam pneumatic process, the bottom of the manonry finaling about tweek feet below low water, and about twenty-five feet of manonry having to be lead while the sucking of the foundations is in progress.

The masonry will be first-class rock face work, laid in regular courses. The face stones, including coping, will be of grante, which shall be of uniform character throughout, and acceptable to the Engineer. The backing may be of any good, sound innestone.

The piers shall conform in all respects to the plans furnished by the Engineer.

No course shall be less than staten inches thick, and no course shall be thicker than the course below it.

No course shall be less than staten inches thick, and no course shall be thicker than the course below it the thickness of the course, and no face stone shall measure less than thirty inches in either borizontal direction, In general, every third stone of each course shall be a header, and there shall be at least two headers in each course between the shoulders. No stone will be considered a header that measures less than five feet from the face. The headers shall be so arranged as to form a bond entirely through the pure, either by bonding against a face some in the opposite ade of the course, or by bonding with a prece of heaking not less than three feet square which shall bond with a face stone on the opposite ade. In all cases the interior bonding shall be further secured by placing in the course above, a stone of the full thickness of the course, and at less three feet square, over the interior joints. Special care shall be taken with the bonding of the ice breaker cut water, the stones of which shall be so arranged that the face stones are supported from behind by large pieces of heaking.

All joints shall be pitched to a true line, and dressed to one quarter of an inch for at least twelve inches from the face. Beds, both upper and lower, shall be pitched to a true line, and dressed to one quarter of an inch. Joints shall be broken at least fifteen inches on the face. The bottom beds shall always be the full sure of the stoor.

The pounted up stream starlings of Piers II and III, from the footing courses to the small coping at the offset, shall have a fine pointed face, with no projection exceeding one-half unch from the pitch line of the joints. There shall be a draft line three inches wide, around the lower edge of the beling course below the coping, and on the edge of the pointed starlings. The entire coping over the whole pier, and the small copings over the starlings, shall have a rough quarry-face, with no projection exceeding three inches from the pitch line of the joints.

The face stones of each entire course of Pers II and III above the footing courses and below the offset at the top of the pointed startings, shall be doweled into the course below with round dowels of one and one-eighth inchinon, extending six inches into each course. The dowels shall be from eight to twelve methes back

from the face and ax maches on each sade of every joint, the stones of the upper course shall be drilled through before setting after which the drill-hole shall be extended six makes into the lower coarse a small quantity of mortar shall then be put into each bole, the dowel dropped in and driven home, and the hole falled with mortar and rammed. The three courses below the coping shall have the joints boand with cramps of  $g^*$ round from, twenty inches long between the shoulders, the ends sunk three inches into each stone.

The stones in the coping under the bearings of the trusses shall be according to special plans, to be formshod. They shall have good beeks for their entire sue, and shall have a full bearing on large stones with dressed bold in the belting course below the coping.

The stones of the backing shal, have dressed beds. The backing shall generally be of the same thick ness as the face stones, but two thicknesses of backing may be used for one course of face stones, provided no backing as less than twelve inches thick.

All stones shall be sound, free from seams, and other defects, and the quarries shall be approved by the Engineer. All limestone shall be laid on natural beds.

All stones shall be laid in full mortar beds. They shall be lowered on the bed of mortar, and brought to a bearing with a manl. No spalls will be allowed, except in small vertical openings in the backing. Thin mortar joints will not be insisted on, but the joints shall be properly cleaned on the face and pointed in mild weather, the pointing to be driven in with a gaulking mon.

The mortar will be composed of cement and clean, coarse sand, satisfactory to the Engineer, in proportions varying from one to three parts of sand to one of cement, as may be directed by the Engineer, for different parts of the work. When the stone is laud in freezing weather the contractor shall take such precautions to prevent the mortar from freezing as shall be satisfactory to the Engineer.

No material shall be measured, nor included in estimates, which does not form a part of the permanent

The Railroad Company will furnish free transportation from Kansas City or Council Bluffs, or any intermediate point on the K. C., St. J. and C. B. R. K. to the bridge site, for stone actually used in the piers. Free transportation will also be furnished from any approved limestone quarry in Nebrasko, not more that pay miles from Rulo, on a railroad operated by the C. B. & Q. R. R. Co. This free transportation is given on the assumption that the stone is to be cut at the burry, and if the contractors prefer to cut at the bridge site, a charge will be made at the rate of eight mills per ion per mile for the difference between the weight of the stone transported and the finished weight, as lucd in the piers. Any stone transmitted free, and not used, will be the property of the Railroad Company.

The Raulroad Company will furnish cement for mortar, which must be handled from the cars, or storehouses by the Contractor, who will be held responsible for any loss or waste,

The Contractor will be required to furnish all tools of every description and all materials, except cement, and will be responsible for all damages which may occur from the conduct of the work embraced in his contract.

### APPENDIX D.

### RECORD OF SINKING CAISSONS. PIER I.

Е	LEVATIO	ONS OF C	UTTING	Enge					EIEVATI	IONS OF	3 ROUND		Av	rige							WEIG	HTS.					Atk Pri		Reaction	Ner	Surface	Average Weight per so ft	
N. E	w w	5 1		w	k erago	Bunk III 24 Hours		E	N W	SE	s w	Au r	Car	ene nt on of	Water Gauge	Depth Innersed		Casson Iron, C	lonciete	Timber.	Cmb Iron, C	Concrete.		Masonry	. Sand	Total.	Indicated	Cajculated.	due to Ar Pressure.	Weight	(ontact	surface exposed to friction.	KEW AKA
844 12 844 24 837 45 830 830 83 830 64 830 625 03	824 8	15 844 20 843 47 842 56 838 37 834 76 831 47 830 83 834 83 834 83 838	00 8.4 .86 84 .17 84 .25 83 .70 83 .70 83 .78 83 .30 87	4.18 4.12 12.44 7.92 4.05 11.67 10.16 8.00	841.83 837.92 834.47 831.25 830.39 525 13 824 69	2.15 3.61 3.40 3.40 3.40 3.40 3.41	851	1 50 1 50 2 70 3 10 3 10	849.04 851.80 851.20 849.90 850.50 849.00	847.00 851.02 850.29 850.40 050.01 850.60 850.60 850.60 852.60	850 1 853 6 851 0 851 0	550 2 851 0 851 0 851 0 851 0 851	E 00 3 8 8 13 17 17 17 80 1, 17 17 80 1, 13 14 13 14	70	849 90	H G C Ft 4-37 6-83 10 20 14-83 10 20 12-12-12-12-13-13-13-13-13-13-13-13-13-13-13-13-13-	fons 155 155 155 155 155 155 155 155 155 15	255 255 255 255 255 255 255 255 255 255	166 352 578 578 578 578 578 578 578 578	70ns 	rens	2009 412 613 612 719 655 873	Tcn 6 6 6 6 7 8 8 9 9 9 9 9 9	Tons-	3 24 35 55 66, 93 01	Tens 186 352 583 773 1020 1264 1480 1515 1630 1794 1890	K L. 8 2 75 4 75 6 7 7 9	L s 1.88 2.95 4.75 6.42 7.98 8.44 9.48 11.04	ITONS  179 251 452 611 750 604 903 1057	N I M Tons 404 492 568 653 726 7 I 753 747 833	0 8q ht 160 1047 1500 250 3113 3443 3094 4000	1756 940 725 949 725 495 497 497 497 497 497	Began Concreting at 8 a. m.  Started ur pumps at 8 a. g. m. Started Water Pumps at 8 a. m.  Began building crib. Material passed through, river sand.
814-69 810-46 808-65 506 53 506 53 507 57 804-81 804-77 800-72 798-54 775-60 791-40 791-78 785-55 78	814-0   814-0   816-6   806-6   806-6   805-0   805-0   805-0   805-0   805-0   805-0   805-0   775-0   795-7   795-	99 814 57 50, 53 807 55 805 55 805 55 805 55 803 50 803	7.4 88 82 82 82 82 82 82 82 82 82 82 82 82	44-70 97-65 97-55-52 97	NI4 06 Sto 14 06 Sto 14 06 Sto 14 06 Sto 14 25 Sto 17 25 Sto	1 77 1 77 2 02 1 1 77 0 00 0 02 0 00 0 04 0 02 2 2 54 2 29 1 77 0 06 0 07 0 07 0 07 0 07 0 07 0 07 0	254444	10 10 10 10 10 10 10 10 10 10 10 10 10 1	349.00 849.40 846.40 846.40 846.30 853.40 8553.40 8554.07 8554.07 8554.07 8554.07 8554.07 8554.07 8554.07 8554.07 8554.07 8554.07	857	0 7 6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	00 00 00 00 00 00 00 00 00 00 00 00 00	1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	72	847.70 848.25 847.70 847.40 847.40 847.40 847.40 847.40 846.45 846.25 846.25 846.25 846.25 846.25 846.20 847.40 848.40 848.40 848.40 849.40 849.40 851.40 85	\$ 110 00 00 00 00 00 00 00 00 00 00 00 00	15555 15555 15555 1555 1555 1555 1555	1955 255 255 255 255 255 255 255 255 255	5788×××57885778857788577885778857788577	122 105 120 120 120 120 120 120 120 120 120 120	7 7 8 9 10 10 10 10 10 10 11 11 11 11 11 11 11	1007 1154 11530 11530 11530 11735 1735 1735 1735 1735 1735 1735 173	9 10 10 10 10 10 10 10 10 10 10 10 10 10	*50 50	55 5 4 5 4 4 4 4 4 4 4 4 4 5 5 5 5 5 5	2035 2737 2407 2568 3047 3146 3148 3349 3349 33780 4176 425 4374 4533 4546 4765 4765 4765 4765 4765 4775 4743	11 13 15 17 5 10 5 10 5 10 5 10 5 10 5 10 5 1	11-43 13-72 14-15 14-15 18-0-4 17-58 17-58 17-58 18-7-5 18	1084, 1759, 1518, 1508, 1778, 1778, 1779,	646 (coop (coop)	4430 4900 4900 5177 5734 5752 7052 7053 7135 7135 7135 7135 8277 8449 8277 8775 8775 8775 8775 8775 8775 8775	477 4095 585 4378 77 78 836 445 447 447 447 447 447 447 447 447 447	Sand. Reached class. $Hew pressure \ off \ to \ g \ lise, \ in settling. Began exercation for test \ gri. \\ Referred to bottom \ of "Test P,0" column \ H=08,7 ft., \ L=9,972 \ lise, \ M=9890 \ model of the property of the$
787.85 787.14 786.15 786.15 786.05	787.5 787.5 786.8 785.8 785.8 785.7	\$\$ 787 \$1 787 81 787 82 786 82 786 76 785	.74 -3 .76 78 .05 74 .04 78 .04 78	37 50 51 53 50 85 85 80 85 72 85 72	757 60 707 10 705 15 705 15 135 65 135 65	0.00 0.00 0.71 1.00 0.00 0.00	85: 85: 85: 85: 85: 85:	2.30 1.30 2.20 2.20 4.10	854.40 854.60 853.80 853.80 853.80 854.30 854.30	853 to 853 20 852 90 852 20 852 20 852 40	853.2 853.1 852.5 851.8 851.8	0 853 0 853 0 853 0 852 0 852	50 66 66 66 66 66 66 66 66 66 66 66 66 66	5 7 4 7 24 7 24	847.60	60.54 60.1+ 00.65 62.25 62.70 62.92 62.67	155	75 25 25 25 25 25 25 25	579 570 570 579 578 578	234 234 234 234 234 234 234 234 234	13 13 13 13 13 13 13		20 20 20 20 20 15 15	50	613 637 635 863 863	4743 4743 4747 4745 4772 4717 4739 4739	25 34 36 26 25 25 25 25 24 36	16 37 26 1 16 01 20 24 26.43 17 13 21 22 17 11 26 96	1507 2455 1418 2454 2555 2584 2593 2593	1136 1145 1360 2240 1157 1157	9403 9403 9423 9413 9413 9528 963	478 478 40. -77 403 447 403	$\begin{array}{llllllllllllllllllllllllllllllllllll$

### RECORD OF SINKING CAISSONS. PIER II.

nte-	Е	LEVATION	es of Ct 1	TING ED	GB			ELEVA	TIONS OF	GROUND		Average							W	BIGITS.					Ans I	RESS, RE				Average Weight	
1	-		1			Sunk in 24 Hours	_				åverige of to	Per e	Water t-at g	D pin Immersed.		Caussa i			Crib	I c	ir ick Mar	oner 6	2004 1	Varer T in	Increased	Colo lote	he ac jon C e 'c Asi Pressure	W+ g	Surface Contact	persq ft on surface exposed	Remarks
	N E	N W	5 Б	s w	Average	11. 0.7	NE	N W	S. E.	s w.	of 12 Soundings	Cabason			Timber	Iron C	Concrete	Timber	Iron. C	lonerete el	art dus	10417 8	Jan 1		Thate trea	CHESING	4			to friction	-
(eb) (1) 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	843-32 838-65 837-79 837-75 837-75 837-75 839-77 829-77 829-77 829-77	543 97 548 65 557 75 557 15 550 25 550 25 550 25 550 25 550 25	829 89 829 79 821 32 820 27	\$13 3" \$35 65 \$37 79 \$37.15 \$37.13 \$35.13 \$35.30 \$30.27 \$30.09 \$22.23	843 32 848 65 837 79 837 15 837 15 830 08 830 06 830 06 830 06	7 50 - 67 - 67 - 6 60 -	5+1 3-1 5+3 7-1 5+3 7-1 547 8-8 847 00 847 00 846 8-6 8-7 10	843.30 841.50	217 84 217 84 21	847.50 847.50 847.50 545.71 545.71 546.40 637.60 848.00	17 7 5 d 7 7 5 d 7 7 5 d 7 7 5 d 7 7 5 d 7 7 5 d 7 7 5 d 7 7 7 5 d 7 7 7 7	1 96 11 63 11 63	847 65 847 65 847 66 847 66	3 08 7 80 8 02 9 15 0 31 16 37 16 69 17, 27 17 61 13 3 1 12 27 23 22	10 s 77 177 177 177 177 177 177 177 177 177	Ions 37 32 32 32 32 32 32 32 32 32 32 32 32 32	Tens 3,77 515 775 764 856 856 856 856 856 856 856 856 856 856	To s	Fons.	T ns To	77777777777799999999999999999999999999	ons	Ion .	Tons Tons 2.4 2.6 2.16 2.15 7.13 7.3 7.4 4.9 4.9 4.9 3.0 7.4	K L .	L Lbs		A I M Toms  47.  682 688		2054	Carson balt at elev. \$50.88. Began lowering at 8.30 a. m. Began concreting at 12.40 p. m. Stopped at 3.30 p. m.  Put on air at 645 p. m. Started water pumps at 8.15 a. m.  Fegan balding crib in P. M. Stopped water pumps 410 a.m. Resumed work in H. at 5. m., 1  Resumed work in Pier II at 6.45 a. m. Started water pamps at 6.55 a. m., 5 topped water pumps at 5 a. m. Resumed work in Pier III at 6.45 a. m. Working in Pier III.
lar 2 3 4 26 78 90 12 3 11 15 6	820.52 62 50 820 48 820.48	50 3, 820 3, 830 43	820.28	820 28 . 10 11 820,24 820,24	820 35	0 63 0 63 0 90	846.56 847.16 843.06 845.06	2 838.20 2 838.20 3 820 50	846.60 846.60 846.70 847.70 847.70	846 20 \$47 33 \$47 40 847 40 847 00 843.70 \$35 50 \$36 00	840 32 631 04 640 32	25.29	347 35	26.96 77 12 7, 13 19.35	77 177 177 177 177 177 177 177 177 177	32 37 32 32 32 32 32 32 33 34 35 37	\$56 \$56 \$56 \$56 \$56 \$56 \$56 \$56 \$56 \$56	92 92 6 6 116 116 116 116 116 116 116 116	6 ( 6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	75,2 1033 1017,4 15440 13446 13446 13446 13446 13446	999999999999999999999999999999999999999										All work suspended -ice too weak  Ice went out at 5 p. m.
75	815.38 515.06 505.06	817 815 815 807 90 803 804 14 804 14 804 14	815.04 815.04 815.04 808.38 7 12 12 12 12 12 12 12 12 12 12 12 12 12	817-33 5 67 807-05 804-59 804-59	815-51 813-86 51-38 805-74 805-74 804-61 804-32 804-32 804-32	3 20 51 65 7 73 93 93 9 55 1 7 75 1 30 1 8 9	824 7 825 7 876 8 875 4 876 9 826.9 828.3	824 40 825 40 827 40 827 80 814 80	819,60	837 20 837 20 30 7 3, 40 839,40 31,40 829,30	85° 47 876 25 576 25	25 60 22 17 24 23 25 00 12.48 6 5 5 16.57	850 50 851 15 850 20 849 40 849 95 852 20 854 00	30 35 34-13 34-69 35 59 35 77 40-30 42-06 47-33 49-39 50-95	177 177 177 177 177 177 177 177 177 177	32 32 32 32 32 32 32 32 32 32 32 32 32 3	8566 8566 8566 8566 8566 8566 8566 8566	116 116 16 175 145 145 175 178 178	777888	1346 134 1346 1429 1503 1694 1855 1983 2106 2217 2217 2217 2217	7 / 10 mm		59 116 172 116 156	178 2741 177 13 174 300 174 300 174 370 184 370 184 385 185 370 184 385 185 370 186 370 186 370 187 387 381 387 381 390 381 390	16 7 16 7 20 11 21 5 12 5 22 5	14.76 15.00 3.3 16.35 17.39 17.43 18.19 10.00 2.3 2.1.81 22.05	1767 1767 1767	1165 353 4074 1563 1074 1752 1360 134 15	1960 2 145 3904 3458 3133 3133 3054 1004 2646 2678	1510	Sand. Plumps stopped at 10,30 p. m.
pr - 1 3 4 5 4 . 2 9 0 1 1 1 1 4 5 6	800.35 794.87 792.45	NO. 11 803.78 30 A NO. 31 797 93 794 97 792.76	803,94 802,79 802,77 77,77 77,77	799-70 799-70 799-70 798-72 798-72 798-72 798-72	804 10 804 10 803 30 801 00 800 04 799 47 798 27 798 27 79 67	0 17 0 18 72 30 6 6 70 3 41 1 3 01	820.4 823.9 825.7 825.1 825.5 826.6 826.6 826.6 826.6	5 816 5 821 70 5 821 70	731 30 522,70 513 60 521 60 513 60	\$25,50 \$25,70 \$25,70 \$25,10 \$25,10 \$28,00	833 .3 73 .0 73 .0 73 .0 73 .7 826 23	15 64	857 50 351 75 850.90 851.15	+30 +845 4845 450 510 517 513 5443 563 57-33 57-33 57-47	1-77 177 177 177 177 177 177 177 177 177	32 32 32 32 32 32 32 32 32 32 32 32 32 3	75575 75776 75776 75776 75776 7576 7576	77.58 17.58 19.55	11	3,16 1500 2500 2500 2500 2500 2500 2510 2810 2810 2810 2810 2810 2810	1.4	74 234 333 400	276	365 3887 356 367 337 4074 337 4074 337 416 34 423 331 416 362 416 362 416 362 416 362 416 362 416 362 416 362 416 362 416 362 416	21 0 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1	15 5 22 5	2350 2350 2361 2361 2514 2514	1576 160, 1835 92 16 183 30, 51 15 2162 2115	2646 24 3 35 1 7 342 1 35 3 42 10 46 3 46 3 46 3	1257	Pumps started at 720 a. m. Sand, gravel and balls of clay  Finished cnb. Pumps stopped at 640 p m and started at 10.50 p m  Fumps stopped at 920 p m. Started at 12.45 p m.  Finished concerting cnb. Started at 12.45 p m.  Pumps stopped at 925 a. m.  Pumps stopped at 925 a. m.  Pumps stopped at 93 a. m. and an shed at 5.0 p m  Cuseoff of V.  Wording in 19.11.

PIER II. CONTINUED.

É	ELBVAT	TIONS (	OF CUIT	ING ED	E	Su.			ELEV/	ATIONS OI	GROUND		Assange Posse trut on		Dej Immersed		Cass			(mh	A KI S	A.r L. l. as	To comme	Daniel 1	Water. Total.		Calculated	Re d n due co A r Proper	Nor Weigit	S sface in Corties		Rivarks
N B	. N 1	J.	SE	8 W	Averag		,	NE	N W	ь Е	S W	of 12 Southings	Causs.,	(,		Timber.	Iron 'C	oncrete.	Timber .	Iron (	Concrete.		insonry.	BRIDG.	water. Total.	- K	L	м	\		Exposed United	
				791 30 771 32				33 0 36 00	5 Ny 10 531 TO	037 10 038 70	83° 10	83+ 02 837 35	F-C F.	850.10 5,0.20 849.85 849.70 549.30 849.30 849.30 848.55	58 7)	. 16.5 177 777 777 177 177 277 - 277	Tons 32 32 32 32 32 32 32 32	Lons 856 856 856 856 856 856 856 856 856 856	1 ns 190 198 198 198 198 198 198	13	28 ( 28 ( 28 ( 28 ( 28 ( 28 ( 28 ( 28 (	15	Tons 4So 510 510 540 540 540 540 540	len.	Tons Tuns.	Lis	Los.	L×A T n,	I N T ns	S <sub>4</sub> Fi	N O Lbs	Working on Pier III.
791.27	791	40	791 13	791 26 791-23 791 35 790 09	791 3 191 1	5 00	2 82	29 ro 29 ro 33 yo ,	827.10 827.10 831.90	829 10 829,60 829 90	833.10 833.90	830 60 832 15	36 19 1, 36 +10	848 75 848 45 848 15 848 10 847 90	.57.17 56.89 56.86 57.85	.77 .77 .77 .77	3° 3° 3° 3° 3°	856 856 856 856 856	ながんだい	13	2810 1810 26 1810 26	15 22 72 23	540 540 540 540	+13 ++2 50+	75 5377 240 5330 7 5371	26 26 25 5 26 5	24-73 2+ 61 2+ 59 15 02	2628 2626 2626 2626	*694 **04 2 53	563° 5744 6100	956 941 993	Put on air at 7,00 p. m. Set up clay hoist and began working it at 6 p. m (1:)
78, 42, 787, 71, 787, 787, 787, 787, 787, 787,	787-786-6 787-786-6 786-786-786-786-786-786-786-786-786-786-	738 79 6 79 79 79 79 79 79 79 79 79 79 79 79 79	787 - 94 787 - 534 787 - 534 787 - 534 786 - 85 785 - 51 785 - 51 785 - 51 785 - 51 785 - 51 785 - 51 785 - 51 787 - 50 787 - 50 787 - 50 777	787-48 787-48 785-46 785-96 785-96 785-96 785-96 783-76 783-76 783-76 783-76 783-77 776-98 777-776-98 776-98 776-98 776-98 776-98 776-98 776-98 776-98 776-98	787 - 587 -	1 3 4	5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	か、 * * * * * * * * * * * * * * * * * * *	333 50 833 40 833 40 834 83 40 834 70 834 70 834 70 834 70 835 70	\$50,000 to	833 400 0 400 0 833 400 0 100 100 100 100 100 100 100 100 1	834-44 835-135-135-135-135-135-135-135-135-135-1	50,757,770 450,776,845888 5 7 7,8840,8623,976,845888 5 7 7,8840,8623,976,845888 5 7 7,8840,8623,976,8640,8643,8643,8643,8643,8643,8643,8643,8643	517 50 517 50 517 50 517 50 517 75 517 75 517 75 517 75 517 75 517 75 519 75 51	88,89,9430,746126,7746,90,9430,7746,90,9430,746126,7746,90,9430,746,90,9430,9430,9430,9430,9430,9430,9430,9	77 177 177 177 177 177 177 177 177 177		\$\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	\$\$\$\$\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\		\$100 \$200 \$200 \$200 \$200 \$200 \$200 \$200	23	540 540 540 540 540 540 540 540 540 540	55 100 60 10 60 10 60 10 10 10 10 10 10 10 10 10 10 10 10 10	206 53 pc 43,6 43,6 43,7 5,4 5,5 5,5 5,5 5,5 6,5 1,5 2,5 5,5 5,5 6,5 1,5 2,5 3,5 1,5 2,5 3,5 1,5 1,5 1,5 1,5 1,5 1,5 1,5 1,5 1,5 1	790 26 25 5 25 5 25 5 25 5 25 5 25 5 25 5	5 13 15 15 15 15 15 15 15 15 15 15 15 15 15	265.5 265.5 27.5 27.5 27.9	2107 2113 2140 2140 2140 2140 2150 2160 2160 2160 2160 2160 2160 2160 216	63 6358 6495 6495 6495 6495 6495 6495 6495 6495	10	Clay hoss broke down at 5 a . r Sickin 3 or 1  Clay hoss started up at 10.40 , h.  Let in section supply shaft—Stopped clay horst from 7:25 a. n. to 6.45 i Stopped clay horst from to 20 a. m. to 11.40 a. m.  (a)
765.95 765.06 765.45 765.47	2 765 767 4 766 7 766 7 766 5 765 6 765 7 765 7 765	7,76 5,59 5,51 6,05 5,85 5,86 5,42 5,42	766.0, 765.75 765.59 765.58 765.58	770 46 768 78 768 78 768 78 768 78 766 72 766 62 766 62 765 63 765 53 765 51 —	768.7 768.8 766.6 766.6 766.6 765.5 765.4	8 0.5	8 83 8 83 8 83 8 83 8 83 8 83 8 83 8 83	29.15 30.80 31.60 31.00 29.00	833.15 831 80 832.60 532 00 831 00	835 60 835 60 835 00	834 15 834 15 833 80 834 60 837 00 837 10 837 10	833.68 832.68 832.08	64 +5 64 +5 65 +0 67 83 67 64 66 80	848.80 849.15 849.05 849.30 849.45 850.50 851.45	79 42 80 62 81 31 52 37 85 65 71 88 6	177 177 177 177 177 177 177 177 177 177		\$ 10 C C C C C C C C C C C C C C C C C C	\$ 55 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5		25 10 25 10	22 23 23 23 23 23 17 17 17 17 17 17	1 01 10 1255 1335 1335 1335 1335	1110 153 1148 1218 1103 1107 1255 1263 1277 187	10 (03) 10 (08) 10	37.55 37.55 37.55 36. 35.5 32.	33 65 34 36 34 87 35 17 35 63 35 77 36 64 36 56 37 10 37 37 37 37 38 33	35/3 3609 3773 3797 3005 3040 3044 3964 3990 4023	3 24 3 '-4 3 04 1 5 -4 1 7 50 3 3 7 10 1 3 2 4 1 3 2 4 1 3 2 5 1 3 2 4 1 3 2 5 1 3 2 6 1 3 6 1 5 1 5 1 5 1 5 1 5 1 5	88855 8875 9032 8869 9396 9112 9141 9161	703 700 724 730 735 735 770 7.5	Stopped clay host from a plot to a plot to a plot to 6.45 p. Stopped clay host from a plot to 6.45 p. Stopped clay host from a plot to 6.45 p. Stopped clay host from a plot to 6.45 p. m. Clay 4 from 1.65 p. m. Finched assign cay host at 1.45 plot to 6.45 p. m. Finched assign cay host at 1.45 plot to 6.45 p. m. Finched assign cay host at 1.45 plot to 6.45 p. m. Finched assign cay host at 1.45 plot to 6.45 p. m. Finched assign cay host at 1.45 plot to 6.45 p. m. Finched assign cay host at 1.45 plot to 6.45 p. m. Finched assign cay host at 1.45 plot to 6.45 p. m. Find plot to 6.45 p. m. Find elevation of cutting older 765,47.

### RECORD OF SINKING CAISSONS. PIER III.

e I	Elevati	IONS OF CUT	rring Ei	nge	Sunk		ELEVA	IIONS OF	Gr. No		Average		Denth					Coh	WEIGHTS						Ata Pag	bot Rai	Reaction	Net	Surface	Average We sh	RIMARA
	N W	SE	s w	Averag	Hours		N W	5 1.	5 11	Average	Casson	Gage	Depth lin nersed		In n				Concrete	Lock Sair.	Masonry	Sand	Water, Is	co. Ir	nd.cated. (	Calculated.	Reaction due to Air Pressure	Weight	Contra	Surface Expessor to fraction	NI WORKS
1 8,2 10 833 30 6 830,70 7 818 80	5 542 5 533, 5 530 5 5 52)	80 846.80 10 8,2 10 30 833 30 70 830 70 829.80 60 838.90	842.1 833.3 830.7 829.8	0   842.t 0   833.3 0   830.7 0   829.4 829.1	0 4 70 0 8 80 0 2 60 2 0 28 0 0 31 0 0 00		831.60	- 830.10	- 829 to	 830 w	E—C P <sub>1</sub> C	6 842.20 842.05 842.05 842.70 841.80 843.00 842.60	9.40 9.40 11 20 13 50 13 50	Fons 177 177 177 177 177 177 177	To 15 34 34 34 34 34 34 34 34 34 34	Nons 89 1.+ 114 205 43+ 605	Tons.	I ons	Tens	Tons. 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	Ions	l'ons	5 9 11 15	1 218 218 118 313 341 341 343 607 836	K Lbs	L Lbs.	L×A Tons	I M Tens	O Sq Ft.	P N÷O Lbs	Casson Militatee, by656 Began lowering casson in P. M. Began concreting in A. M
\$ 86.5 years \$ 86.	875.0 824.0 824.0 824.0 824.0 825.0 821.0	71 Ang 66 62 339 Ang 62 339 Ang 62 339 Ang 62 32 Ang 62 An	825 - 1	27 / 20 / 20 / 20 / 20 / 20 / 20 / 20 /	0 007 0 0 1 155 0 0 0 22 0 0 0 0 25 0 0 0 25 0 0 0 0 0 25 0 0 0 0 0 25 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	247 /3 520 - 5 530 - 5 530 - 5 531 - 5 533 - 7 533 - 7 530 - 8 531 - 4 531 - 4 532 - 8 533 - 4 534 - 4 534 - 4 535 - 8 537 - 6 538 - 8 539	826 30 827.80 827.80 827.80 827.80 827.80 827.80 821.80 82	87,70 824,70 826,00 830,00 830,50 831,80 857,40 857,40 857,40 857,40 857,40 857,40 857,40 857,40 857,40	797 77 A A S 257 A D D D D D D D D D D D D D D D D D D	827 90 42 879 90 42 879 870 42 889 71 889 81 833 43 933 43 933 43 933 43 934 95 826 61 82 38 826 63 826 63 826 63 826 63 827 63 827 63 828 63 83 83 83 83 84 84 84 84 84 84 84 84 84 84 84 84 84	00 +3 00 53 00 97 00 17 1 83 1 7 3 1 5 3 5 3 5 3 7 3 1 0 00 10 17 10 10 10 10 11 10 12 10 12 10 13 11 14 11 15 16 25 17 17 17 18 1	841-99 841-79 841-79 841-79 841-79 841-79 841-79 841-89 84	14.58 12.95 16.68 12.95 16.68 12.95 16.68 12.95 16.68 12.95 16.68	177 177 177 177 177 177 177 177 177 177	*********************************	605 605 605 605 605 605 605 605 605 605	\$ 5 5 16 1 31 32 32 32 32 32 32 32 32 32 32 32 32 32	100000000000000000000000000000000000000	.10 300 530 530 530 700 707 707 707 707 707 707 168 251 153 153 153 153 153 153 153 153 153 1	77777 (858 ) 9999 999 1990 100 0 100 0 100 0		11 1 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1	11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	8 \$ \$ 4 \$ 5 \$ 6 \$ 6 \$ 6 \$ 6 \$ 6 \$ 6 \$ 6 \$ 6 \$ 6	5666797889999999999999999999999999999999	6.90 7.51 7.54 7.87 10.61 13.20 13.142 2.08 13.142 2.08 16.07 17.79 18.88	743 865 859 027 1054 1116 128 1186 1186 1186 1186 1186 1186	791 413 566 653 769 776 730 730 730 730 730 730 730 730 730 730	1, 201 1, 201 24, 8, 18, 2032 2037 2037 2037 2037 2037 2037 2037		Began building crib.  Put on air at 10 a. m. 4 lbs. Started water pumps at 4 p. m. Pumps stopped at 9.35 p. m. Sand  Pumps started at 15.55 a. m. Pumps started at 15.55 a. m. Pumps stopped at 11 a. m. Plumps stopped at 11 p. m.  Pumps stopped at 11 p. m.  Pumps stopped at 26.55 p. m. and started at 4.50 p. m. 100 p
786.6.1 784.88	7 44 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	27 790-86 778 6.6 64 786 6.6 64 7	789. 789. 789. 789. 789. 789. 786. 786.	786.6 786.6 786.6 786.6 786.6 786.6 786.6	77.66	54 - 6 540 - 0 541 - 3 541 - 3	836 30 836 30 836 30 836 30 837 40 837 40 837 40 837 40 837 40 837 40 837 40 837 40	831.60 830.60 830.60 836.80 836.80	835-10 835-10 33-4-60 33-4-70 34-70 34-70 34-70 34-70 34-70 34-70 34-70 34-70 34-70 34-70 34-70 34-70	836 61 836 .84 .22 .23 .23 .23 .23 .23 .23 .23 .23 .23	3. 3. 3. 3. 3. 3. 3. 3. 3. 3. 3. 3. 3. 3	8.6 os 846.75	49.64 54.41 55.51 50.51	177 25 177 177 177 177 177 177 177 177 177 17	林林是在野林林 *** 在外面提供的 ************************************	77888000000000000000000000000000000000	17.11 12.15 18.15 13.15 18.15	11 11 11 11 11 11 11 11 11 11 11 11 11	206 1048 113 1298 141 151 151 151 151 151 151 151	11122 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	61 446 3 7 7 138 473 527 473 523 633 633	423 433 433 433 433 433 433 433 433 433	147 3 3 147 147 147 147 147 147 147 147 147 147	700 850 700 850 700 850 844 444 443 750 850 850 850 850 850 850 850 850 850 8	10 10 10 10 10 10 10 10 10 10 10 10 10 1	21 447 97 4 4 5 3 3 4 4 9 3 4 4 5 3 5 5 5 5 5 3 4 4 5 3 5 6 5 7 5 5 5 5 3 4 4 5 7 7 7 7 5 5 7 7 7 7 7 7 7 7 7 7 7	1998 2592 2447 1078 2600 2600 2600 2600 2600 2600 2600 260	1436 1557 1034 1647 1743 1854 1860 1000 1000 1000 1000 1000 1000 1000	5535 5443 5539 5639 5736 5736 5736 5736 5736 5736 5736 5736	50% 5560 5560 5560 5560 5560 5560 5560 5	Pumps stopped at 0.35 a. m.  Pumps stopped at 0.35 a. m.  Pumps stopped at 4.9, m. and started at 7.05 p. m. Sand and mud.  Pumps stopped at 2.50 a. m. Fumps stopped at 2.50 a. m. Sacking c.  Causson on clay  Finished concreting crib at 2.30 p. m.  Began Laying maximity in P. M.  Started city. Boost in p. m.  Started city. Boost at 2.40 p. m.  Clay boost from p. m. Started at 12.00 p. m.  Clay boost from p. m. Started at 12.00 p. m.  "" 7.30 a. m. Stopped at 4. p. m.  Started city. Boost at 11.45 a. m.  Finished city. Boost at 11.45 a. m.  Stopped city. Boost at 11.45 a. m.  Stopped city. Boost at 12.15 p. m.
** 750 36 * 750 36 * 750 36 * 779 86 * 777 86 * 777 66	6 7 % 5 7% 6 7% 6 7% 8 777 8 777	31 7.0 3: H 100 3:	780. 780. 50 777. 777.	27 700.1 16 780.1 11 70 779 1 65 777.0 49 7.7	12 1 27 10 0 2 10 0 0 12 0 3 17 1 15 15 0 0 16 0 12	840 84 030 54 534 64 541 34 840 44	834.80 834.80 635.00 835.30 834.60	*15 50 *15 50 *15 50 *13 50 *13 50 *11 50 *11 50 *11 50	832.40 832.40 832.70	835 28 535 4	54.86 54.86 57.90 57.70 58.19	846.85 846.40	668 663 06 05 (C 13 69.23 61.32 62.74 69.53	177 177 177 177 177	3+ 3+ 3+ 3+ 3+ 3- 3- 3- 3-	830 830 830 830 830 830	8 . 76 198 198 198 198 198	13 13 13 13 13 13 13 13	2541 2541 2541 2541 2541 2541 2541	20 70 70 70 70 70 27 70 20	633 613 633 633 660 662 735	814 814 817 837 879 891 905	275 5 245 5 245 7 275 5 284 5	517 1515 1515 1515 1616 1618 1618 1618	30. 30. 30.5 30.5 31. 30.5	2h 7h 15 8; 16.8; 28.99 29.95 20.95 30.08	3078 3078 3079 309 3102 3212	2446 2435 2435 2420 2440 2435 253	7736 7737 8040	631 630 630 62 590 607 604 635	Stopped clay hoss at 330 p. m. and started at 630 p. m. 1 CO p. 1. 1 CO p. 1

### APPENDIX D. -Continued.

PIER III.—CONTINUED.

E	LEVATION	NS OF CUT	TING ED	)B	NuBh J <sup>TI 24</sup> Hours	(	ELEV	TIONS (	F GROUNE		Average Leve tration	Water	Depth Imnersed		Caisson			V Crib.	RIGHTS	Air					AIR Pas	85 MR.	Renetion due to Arr	Net	Sartace n Connet	Average Weight per on H	Remarks.
N E.	N W	8 E	9. W	Average		N. E.	N. W.	S. E	s. w	Average	CI CIL	(rauge	THE HE ISOG			Cancrete	Timer		Сонстеца	M 18 1	Masonry	Sand V	Water I	otal I	Indicated, C	Calculated.	Pressure	**************************************	Conact	exposed to frenon	
70.70 70.72 70.71 69.36	774 01 774-62 77-4 (173 50 773 3 771 57 771 57 771 57 770 64 770 64 770 64 770 64 770 64	774 73 773.63 773.44 771.6 772.09 772.03 771.96 770.70 770.70 770.70	774 60 774 60 774 60 773 49 773 31 772 97 77 98 77 63 770 63 770 63 770 63	774.67 773.56 773.37 772.07 7;2.0; 7;2.0; 7;2.0; 770.67 770.67 770.67 770.67	16001 001 1100 000 000 1130 000 133	835 to 835 to 835 to 835 to	\$35.90 \$3	830 200 200 200 200 200 200 200 200 200 2	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	836 - 13 837 - 14 837 - 14 837 - 14 837 - 14 837 - 14 837 - 13 837 - 14 837 - 15 837 - 15 836 - 37 836 - 48 836 - 48 836 - 48 836 - 14 836 - 14 836 - 14 836 - 14 836 - 14 836 - 15 836 - 16 836 - 16 837 - 16 836 - 16 837 - 16 837 - 16 836 - 16 837 - 16 837 - 16 837 - 16 837 - 16 838 -	67 35	846.60 846.90 849.90 849.40 846.30 846.30 846.30 847.70 847.60 847.60 847.60 847.60 847.60	G C Pt Pro 68 70 68 70 67 17 70 71 70 71 70 71 70 71 70 71 70 71 70 71 70 71 70 70 70 70 70 70 70 70 70 70 70 70 70	Tons77 177 177 177 177 177 177 177 177 177	Tans. 34 34 34 34 34 34 34 34 34 34 34 34 34	24	Tons 4 7 3 5 5 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5		Tons 3541 2541 2541 2541 2541 2541 2541 2541 2	Tons 20 21 21 21 22 22 22 22 22 22 22 22 22 22	807 806 916 916 916 916 946 707 1067 1140 1539 1239 1549	Tons 164 0 5 1040 1055 1177 1169 1177 1169 1170 1218 1218 1218 1217	180 5 284 3 3 285 6 6 265 6 265 6 6 265 6 26	Cons. 6 40 7 10 25 10 4 10 25 10 4 10 25 10 4 10 25 10 4 10 25 10 4 10 25 10 10 10 10 10 10 10 10 10 10 10 10 10	K Lbs. 31 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	L hs 578 45 57 58 45 57 58 58 58 58 58 58 58 58 58 58 58 58 58	M L×As 3264 3255 3315 3315 3360 3371 3435 3435 3435 3435 3455 3556 3556 3615 3615	N 1 M Tons 2594 2652 2713 2720 2738 3744 2713 2550 393 3037 3039 3057 3057 3051	6, 8376 8376 8376 8554 8554 8659 8659 8659 8659 8659 8659 8659 8659	63. 62. 630. 647. 668. 681. 674. 674. 674. 676. 681.	Clay Stopped clay hosts from 3.30 p.m. to 7.45 p. m. Gravel and Coarse San Strated at 5.50 p.m. Strated at 5.50 p.m. Working in P. II. Strated at 8.40 m. and stopped at 6.30 p.m. Stopped at 8.45 a.m. and started at 3 p. m. Topped at 8.45 a.m. and started at 3 p. m. Topped at 8.45 a.m. and started at 3 p. m. Topped at 8.45 a.m. and started at 3 p. m. Topped at 8.45 a.m. and started at 3 p. m. Topped at 8.45 a.m. and started at 3 p. m. Topped clay host at 5 a. m. and started at 12 m. Clay
	man ii	768.00 767.50 767.47 767.47 767.48	765 % 767 46 *6" 45 767 45 767 45	768.00 767.50 767.49 767.48 767.48	0 14 0 0 0 0 1 00	836.00 835.70 835.70 836.00 835.70	832,60 833,30 831,40 832,50 832,70	834 2 36 3 36 5 836.3 836.2	0 \$33 40 0 \$33 40 0 \$35 40 0 \$35 60 0 \$35 70 0 \$36 30	834-45 834-95 835-79 835-82 835-82	67 64 67 86 67 86	847.25 847.20 847.35 817.40 847.40	79:25 79:70 79:70 80:00 80:07 79:92	177 177 177 177 177	3+ 3+ 3+ 3+ 3+ 34	830 830 830 830 830 830 840 840 840	10 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	19	15+ 25+ 11+ 25+ 15+ 354+ 11+ 2541	27 27 27 27 27 27 27 27 27 27 27 27 27 2	125 / 125 ) 125 ) 125 / 125 ) 125 ) 125 ) 125 )	212	3gi) 6	KK <sub>9</sub>	35	34 25	ħφX	3025	8,88	673	Stopped clay hoss at 2.55 p. m. Cut off air at 5'55 p. m Work st-spended. River breaking up lee went out at 4.50 p. m
767 54 7 ** 2 *	767.45 767.14	767 - 44 17 - 23	767.42 767.21	767.46 767.21	ים נ							845 90 647.00 847 15 940 20 846 00	75, 65 80.95	177 -	34 3+ 3+ 34 3+	830 830 830	なるないな	13 13 13 13	2541 2541 2541 2541 2541	20	1256 1256 1256 1256	2 1" 267	848 G	d by	35.5 35.5 35.5 35.5	34 4 <sup>5</sup> 35.05 35.04 35.04	3f × 2 374.2 374.2 374.2	2487 2576	## Z Feh	1.8	Put on air at 10.40 a, in. Started clay hoist at 3:25 p. in. Current too swift for so andrings.
766 23 -06 16 765.71	766 .* -66 1. 765.67	766.35 766.77 765.78	766 36 766 35 765.78	766.28 766 ° 765.74	0 07 0 17				-	-	-	850.45 65.45 850.60 851.15	84 17 83.99 84.86	177 177 177	34 34 34 34 34 34	830 830 830 830	02. 7. 7. 7. 7. 5. 5. 5. 5. 5. 5. 5. 5. 5. 5. 5. 5. 5.	13	2541 2541 2541 2541 2541 2541 2541	32 22 33 16 6	1256 135 125 125 125 125 125 125 125	375	1040 6	443	* ,	36 42	3589	2554	5031	101+	Cut off air at 450 p. m. Took boat subore to coal up. Put on air at 3 pp. m. Started day host at 6 p. m. Stopped chy, host at 1140 p. m. Cut off air 4740 a. m. Abandoned P.er on account high water Working in P.er-II
765.59	765 53 -	765 64	765.64	765.60	o f4			-	-	-		849.40 849.40 849.40 849.40 854.00 854.00 854.00 854.00	83 80	177 177 177 177 177 177 177	34 34 34 34 34 34 34 34 34 34	830 830 830 830 830	198 198 198 198 198 198	15	2541 2541 2541 354 2541 2541 2541 2541 2541 2541 2541	6 16 16 16 16 16 16 16	1259 1259 1259 1259 1256 259 259			· ,			-				" Pier III entirely submerged.
		764.64 764.63			1 00	- - -	_	- -	AMA.			854.30 853.40 853.40 857.50 851.75 850.40 850.40 849.50	87 In 86,54	177 177 177 177 177 177 177 177	34 34 34 34 34 34 34 34	830 830 830 830 830 830 830 830 830	7554683554 V	13 13 13 13 13 13 13 13 13 13 13 13 13 1	2541 2541 2541 2541 2541 2541 2541 2541	16 16 16 16 16 16 16	1259 259 259 259 259 259 1259 1259 1257		Uyh 6 1086 6		~	-		6,11	5+5+ 5380	3404 3437	
63 01	764-51 764-52 764-52 764-52 -64-3 763-90 763-90	764 64 764 64 764 63 764 63 764 00 763 61	764.66 764.64 764.65 764.65 764.65 764.63	764.60 764.60 764.58 764.59 764.47 763.95 763.57	0 % 0.01 0 12 0 57 0.38	-	_	-	P4	-		849 50 \$48.50 848.50 848.80 849.15 849.80 850.10 649.85 849.70	84-00 84-20 84-57 85-21 85-63 80-25 86-38	177 177 177 177 177 177 177	34 34 34 34 34 34 34 34 34 34 34 34 34 3	\$30 830 830 830 830 830 830 830	157 158 158 158 158 158	13	2541 2541 2541 2544 2544 2541 2541 2541	16 16 16 16 22 22 22 22 22	1759 1759 1759 1759 1759 1759 1759 1755 1755	355	07 6	143 462 463 504 504 528	36.5 36.5 36.5 36.5 36.5	36 +3 - 36.59 - 36 +1 - 37.05 - 37.33	30100 3900 3510 3956 3275 3 1860	2555 2555 2579 2548 2548 2542	503 5116 516 522 538 5387	1013 1000 for 570 5+2 5+4	Put on ar at 335 p. m. Storped from 7.30 a. m. to 9:30 a. m. Storped disk has at 12.5 a. m. St.k. 52.41 f. m. f. to 184.32.01 Januaran 8. 1. 1866 M. (2.2 p. f. 1.2 p. f.)
		763.60 763.62 763.59			2 OJ D UZ	_	-			_	-	14 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	85.33 Name 84.62	177 177 177 177 177 177	1 34 34 34 34 34 34 34 34	830 830 830 830 830	900 198 198 198 198	13	2541 2541 2541 2541 2541 2541	10 11 23 12 27	1259				35 5 31	y . 2	3347	_			Frushed scaling casson at 1 a.m. Cat off air at 8 to a.m.

### RECORD OF SINKING CAISSONS. PIER IV.

Et	COITAVE	is of Cut	TING ED	GE.		ur n I		ELEVA	TIONS C	or Gro	OND.		Avenuge Pene-				-			Wei	GHT5					Ata Pr	- 1844 AE	' N 10 P		Surface	Average Weigh	
× 1	N V	S, I.	s w	Av r	11	74	N E	N W	1 5. 1	. 8	W.	Average.	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	(N me	Depth Immersed		Caisson Iron		Timber	Crib	Concrete	Lock Shift	Assent	Sand.	Foral	Inciented	Calculated	Air Air	Weigh	Contact	Su atte	Remarks
844 69 843 74 842 62 842 61 811 21	844.88 843.78 842.36 842.36 842.36 847.02 837.02 837.05 836.73 833.04 832.51	41 77 11 01 14 03 14 03 15 03 16	845.72 844.01 511 /9 842.99	845- 845- 843- 842- 841- 841- 840- 838- 836- 836- 836- 833-	05 18 18 083 175 175 175 175 175 175 175 175	D Ft 87 00 355 36 00 15 56 17 17 21 37 34	54.3 JO 844.85	844 65 844 65	- 844 8 844 .!	ш	  	E \$47.00 \$47.00 \$44.00 \$46.50 \$46.50 \$45.30 \$45.30 \$45.30 \$45.30 \$45.40 \$45.40 \$45.40	F E—C Ft 1 92 1 93 3-17 4 05 3-75 4 05 5-81 6 9 8 33 50 24 1 26 14.05	846 70 846 00 0+, 80 845 75 845 80 845 40 845 40 845 20 0+5 10 845 20 0+5 10 844 51 0+4 85	H G—C Ft. 0 82 0 32 3.05 2.95 4.00 5 3 6 5,9 8 43 11 54 11.76 14 05	Tons. 155 155 155 155 155 155 155 155 155 15	Tons 30 30 30 30 30 30 30	Tons. 238 285 354 354 354 354 354 354 354 354 354 35	Tons	Tons	Tons.	Tons 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	Tons.	Tons.	I I I I I I I I I I I I I I I I I I I	k 11 2. 3 5 5 6 7	11.8	A A	192 192 193 193 193 193 193 193 193 193 193 193	0 50 F 295 205 205 205 205 205 205 205 205 205 20	P N=O Lbs. - 1246 1335 (360 974 732 507 537 537 456 28,	Started air pumps at 705 a.m. Water pumps at 730 a.m.
817, 24 817, 21 810, 61 817, 62 810, 64 804, 43 801, 90, 798, 798, 798, 798, 797, 89 796, 90, 795, 50 796, 90, 795, 50 794, 67 793, 57 793, 57 794, 57	8 (7 - 49 817 - 43 8 0 3 8 04 - 31 8 04 - 31 8 04 - 31 799 - 89 798 - 74 798 - 24 797 - 91 796 - 95 795 - 47 795 - 46 794 - 05 795 - 70 792 - 70 792 - 70 792 - 70 791 - 30 791 - 30	817-31 817-33 817-33 809-98 806-03 802-87 800-02 7-7-92 798-39 797-92 795-44 795-43 19-27 795-44 795-45 797-92 795-91 792-91 792-91 792-91	817.63 817.63 810.43 806.16 800.15 800.15 799.12 798.91 798.91 798.55 70.00 70	817. 817. 817. 817. 819. 810. 803. 798. 798. 798. 797. 796. 795. 795. 794. 794. 794. 794. 794. 794. 794. 794	45 4 44 4 7 7 4 4 4 4 4 4 4 4 4 4 4 4 4	25 700 6 00 00 00 00 00 00 00 00 00 00 00 00	1	045, 37, 44 84, 57, 44 84, 65, 77, 60 846, 85, 85, 85, 85, 85, 85, 85, 85, 85, 85	544444444444445777544577	3.71 84 54 84 54 84 54 84 54 54 54 54 54 54 54 54 54 54 54 54 54	3 4 15 4 4 15 4 4 3 55 6 36 6 36 6 5 6 7 6 7 6 7 7 7 8 7 8 7 8 7 8 7 8 7 8 7 8 7 8 7 8	845 - 78 844 - 97 855 316 846 44 844 - 53 844 - 53 845 - 19 845 - 51 845 - 51 846 - 51 846 - 51 847 - 51	54 00	844-80 844-86 844-86 844-70 844-70 844-70 844-85 84-85	16.799 33.10 5 7.77 134 55 156 56 56 56 56 56 56 56 56 56 56 56 56 5	1555 1555 1555 1555 1555 1555 1555 155	86 66 68 68 68 68 68 68 68 68 68 68 68 6	4433773777777743377444377443774443744437444437444437444444	85 58 71 11 17 14 14 15 15 15 15 15 15 15 15 15 15 15 15 15	5 5 6 6 7 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	286 309 51, 808 1185 1365 1365 137 1750	7 9 900 00 00 02 02 02 03 03 03 03 03 03 03 03 03 03 03 03 03		178 178 350 350 357 427 427 4447 464 464 464 50 515 525 525 525 525 525 525 525 525 525	105(1111) 117(5) 11111 117(5) 11111 117(5) 11111 117(6) 11111 117(6) 117(7) 117	\$ 1 12 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	7 16 1 18 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	695 557 1135 1135 1145 1145 1156 1156 1156 1156 1156 1156 1157 11	364 159 241 110 77 113 113 113 114 115 115 115 115 115 115 115	1604 1499 1477 1581	971 90 115 256 3399 267 312 313 334 331 335 312 313 313 313 313 313 313 313 313 313	Stopped clay host at 7,20 a. m. Cracked on one side. Clay host repaired at \$15 p. m.
787.70 786.35 786.36 784.73 784.73 782.99 782.99 781.30 780.04 777.95 777.95 777.95 773.79 773.79 773.79 771.61 770.34 770.27 770.27 769.75 768.20 767.38 770.27 769.75 768.20 767.48	777.90 775.87 775.86 773.74 773.74 771.55 771.53 770.28 770.23 770.23	786 36 786 37 784.06 784.56 1784.77 782.77 782.77 1750.67 1770.78 1775.97 1775.97 1773.86 1771.66 1771.67 1771.67	76. 5 78. 17. 182.91 781.12 781.12 771.00 776.00 776.00 773.494 771.89 770.55	784 784 784 784 784 784 784 784 784 784	41 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	01 03 75 03 75 03 75 03 75 00 12 00 14 00 1	**************************************	545, 11 545, 49 545, 45 545, 45 545, 45 545, 45 545, 45 545, 45 545, 55 545, 55 54 54 54 54 54 54 54 54 54 54 54 54 5	844 844 843 843 843 843 843 843 844 843 844 844	**	13 150 13 150 13 150 13 15 15 15 15 15 15 15 15 16 15 15 16 1	811 1.	56.86 58.65 58.657 60.47 61.71 63.33 64.52 66.35 6.374 64.52 66.35 6.374 70.47 70.48 72.24 73.28 73.33 73.34 73.38	844-60 844-75 844-75 844-75 844-75 844-75 844-75 844-75 844-75 844-85 845-85 845-85 845-85 845-85 845-85 845-85 85 85 85 85 85 85 85 85 85 85 85 85 8	56.86 58.33 % 166	1 3 3 5 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	50 50 50 50 50 50 50 50 50 50 50 50 50 5	443 443 443 443 443 443 443 443 443 443	714 21h 11h 12h 232 232 231 250 254 256 272 272 272 272 273 273 275 285 296 10h	13 13 13 13 13 14 15 15 15 16 16 16 16 16 16 17 17 17 17 17 17 17 17 17	2066 1148 2205 2205 2205 2304 2304 2304 2304 2305 2405 2405 2405 2406 2406 2406 2406 2406 2406 2406 2406	15 15 15 15 15 15 15 15 15 15 15 15 15 1		(555 655 657 (581 (581 (710 711, 740 779 831 531 531 531 531 531 531 531 531 531 5	35 3 5 5 5 7 6 4 5 5 7 6 7 5 7 6 7 5 7 6 7 5 7 6 7 5 7 6 7 5 7 6 7 5 7 6 7 5 7 6 7 5 7 6 7 5 7 6 7 5 7 6 7 5 7 6 7 5 7 6 7 6	15 15 15 15 15 15 15 15 15 15 15 15 15 1	21 (60 25) % 25) % 25) % 25) % 27) 45 27 12 3 31, 32 31, 35 31, 3	2 543 7 4 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	1777 1577 1577 1574 1575 148 148 148 148 148 158 168 168 168 168 168 168 168 168 168 16	8133 8424 8417 8417 8617 8617 8617 8617 907,6 90	100 103 117 117 117 117 117 117 117 117 117 11	Raised clay hoist.  Stopped clay hoist at 3 p. m. Sand and gravel. Started water pumps at 7 a. m. Started clay hoist at 8 p. m.  Grave:  South lock crucked at 3 a. m. in corner of inside well. South lock lenking hally. 10 p. in. Cracked on other side. South lock lenking hally. 10 p. in. Cracked on other side. South lock lenking hally. 10 p. in. Cracked on other side. South lock lenking hally. 10 p. in. Cracked on other side. South lock lenking hally. 10 p. in. Cracked on other side. South lock as a m. Well, and with nak south lock as a m. Well, and with nak south lock as a m. Well, and with nak Reached clay on west side.
	766.14 766.13 766.02 765.08	766.13 766.00 765.07	755.28	705.	06 0	93	845 13 845 15 845 15 843 47 843 47 842 65	843 75 843 74 843 844 12 844 08	544.0	00 84 87 84	13.53 12.98 12.98 13.14 12.71	843 88 843 (k) 843 (k) 843 (k) 843.20	77 05 77.50 77.51 77.0 78 07	843.86 843.86 843.86 843.85 843.85 844.00 844.00 844.10		155 155 155 155 155 155 155	30 30 30 30	443 443 443 443 443 443 443 443	303 303 303 303 303 303 303	17 17 17 17 17 17 17 17	3223 3223 3233 3233 1323 3223 3223 3223	22 11 22 16 16 16 16	12	JUN KS	\$183 \$15 \$151 \$151 \$157	35 5 35 5 35 5 35 5 35 5 35 5 35 5 35 5	41 30 33 00 33 42 33 42 33 40	3 7 / 3 501 3 706 3 745	7004 930 911 95 1 151	10070 1 032 11033 1 035 10	\$65 \$60 \$60 \$55 \$51	Took off clay horst Begin concreting at 11 25 t, in Finnhed filing causin at 83 p m Look off arrat 7 30 a m

APPENDIX E.

### TIME, COST AND MATERIALS USED IN FOUNDATIONS. PIER I.

2   1   1   2   2   3   3   4   2   5   5   5   5   5   5   5   5   5	Principal Nig Foreman Foren	ght sman. F	Sub I orenen Te	oes oders	Pressure Mca	Caffeehous-	, Col	offer	Sugar	Candles	Re l'e	mount pas	Wick	Cond for Heritang.	Lansced (11	Dit- Ont	hing En	Da. gineer	The state of the s	Days -	5	4	Horst	Coa for Poolers	Black Dunount	Signat Amount	Tallow Jonous	Vastr C	ont Oit,	Totals for ach Day	Feet Stuk per day	Ma Can ter Hrs	No of orevo	Rev's LAir amps	Remarks
Coast   1	1 6.00 I 1 6.00 I 1 6.00 I 1 6.00 I 1 6.00 I	3 57 3 3 57 3 3 57 3 3 57 3 3 57 3	9.00 2 9.00 2 9.00 2 9.00 2 6.00 2 9.00 2	4.50 2 4.50 2 4.50 2 4.50 2 4.50 2 4.50 2	\$ 54.00 51.87 56.25 56.25 56.25 56.25 56.25 49.50 54.00 8 40.50	2 3 00 2 3 00 2 3 00 2 3 00 2 3 00 2 3 00 2 3 00	00 3/5 00	0.31 0.31 0.31 0.31 0.36 0.36 0.37	5 0 37 5 0 32 10 0.63 5 0.31 10 0.63	41 0 30 0 32 0 14 0 24 0	89 80 81 1.5 85 2.5 88 1.5 80	0.0%	2 0.00)	1 0 33 1 0 33 1 0 33 1 0 3	0.06		- 2	7-30 7-30 7-30 7-30 7-30 7-30 7-30	1 50 2 3 30 0 3 30 2 3 30 2 3 30 2 3 30 2	4.80 2 4.80 2 4.80 2 4.80 3 4.80 3	2 10 1 + 20 2 4 70 2 4 20 2 4 30 7 4 20 2 1 10 2 4 70 3	2.40 4.80 4.80 4.80 4.80 4.80 4.80		82 26 15 7 24.57 7 13 99 4 14 49 6 19 23 5 15 75 1 5 67	16 0 12 8 0.11 16 0 12 10 0 12	S 0.34	1 0 78 5 0 98 4 0 91 4 0 26 6 0 51 10 1.17 4 0.16 3	- 8 0.38 8 - 8 0.31 16	0 11	127.77 133.21 139.71 133.70 116.78 124.93 123.63 95.20	3.45 3.45 3.22 0.86 2.26 3.44 0.00	22 <sup>1</sup> / <sub>1</sub> 22 <sup>1</sup> / <sub>4</sub> 21 12 20 <sup>1</sup> / <sub>1</sub> 23 <sup>1</sup> / <sub>1</sub> 23 <sup>1</sup> / <sub>1</sub> 23 <sup>1</sup> / <sub>1</sub>	38 38 2 38 2 39 40	29 32 28 27 15	
6 0   3 4   6   19 0   7   5 0   13   13   13   14   15   15   13   14   15   15   13   14   15   15   15   15   15   15   15	1 6.00 I 1 6.00 I 1 6 600 I I 6 600	3 3 3 3 3 4 4 4 4 4 4 4 4 4 4 5 5 6 6 6 6 6 6 6 6	9.00 2 9 9.00 12 9 9.00 12 13 00 12 13	4 50 2 4 50 6 5 50 6 5 50 6 5 50 6 5 5 50 6 5 5 50 6 5 5 50 6 5 5 50 6 5 5 50 6 5 5 5 6 6 6 5 5 6 6 6 6	7 61 50 7 61 50 7 61 50 7 61 50 7 7 61 50 87 50	1 3 00 2 3 00 2 3 00 2 3 00 2 3 00 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 3 00 2 2 2 2	2 / 2 / 3 / 3 / 4 / 3 / 5 / 5 / 5 / 5 / 5 / 5 / 5 / 5 / 5	0 32 0 35 0 50 0 37 0 50 0 77 0 50 77 77 77 77 77 77 77 77 77 77 77 77 77	\$ 0.51 0.063	27 0 1 27 0 1 32 0 1 1 2 0 0 1 2 0 0 0 1 2 0 0 1 2 0 0 1 2 0 0 1 2 0 0 1 2 0 0 0 1 2 0 0 0 1 2 0 0 0	36 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	0.30	16 0.10 16 0.10 16 0.10 16 0.10	1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 1 1 0 3 3 1 0 3	1 0 13 2 0 3 3 0 13 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	9 48 55 56 11 01 9 37 9 87 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	-3.50 2 112.00 1 12.00	7, 30   7, 30	\$ 100 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	4 80 0 1 4 80 0 1 1 1 80 0 1 1 1 80 0 1 1 1 80 0 1 1 1 80 0 1 1 1 80 0 1 1 1 1	4 70 2 4 20 7 4 20 8 4 20 8 8 20 8	4 80 - 4	4-47 9-39 1.49 1.69 1.69 1.69 1.69 1.69 1.69 1.69 1.6	7 6 6 19 57 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 59 5 6 6 18 5 7 7 7 5 8 7 7 7 5 8 7 7 7 5 8 7 7 7 5 8 7 7 7 5 8 7 7 7 5 8 7 7 7 7	8 0 11 16 0 .22 8 0 .11 8 0 .1	8 0.34  8 0.34 8 0.50 8 0.34 8 0.34	9 0.59 10 0.65 10 0	0.07 to 0.07 t	5 o 11	149.90 154.94 156.12 169.28 170.62 173.05 173.12 173.05 173.12 170.41 170.41 170.41 170.41 170.8	4-77 4-52 2-14 4-52 2-14 3-17 0-00 0-00 0-00 1-44 0-02 2-25 1-13 2-54 2-29 1-44 0-02 0-00 1-44 0-02 1-25 1-13 0-00 0-00 1-44 0-02 1-00	21 4 24 20 20 20 20 20 20 20 20 20 20 20 20 20	37 36 36 36 35 35 37 - 37 - 37 37 - 37 37 - 37 37 - 37 -	28 22 35 39 36 39 31 41 42 42 42 42 42 24 24 24 24 24 24 24 24	Pump men ruoning clay hos
	6 00 1 3 6 00 1 3 6 00 1 3 6 00 1 3 6 00 1 3	3.33   6 3.33   6 3.33   6 3.34   6 3.33   6 3.33   6	19.50 2 19.50 2 19.50 2 19.50 2 19.50 2 19.50 2	5.00 5 5.00 5 5.00 5 5.00 5 5.00 5	135 00 137.50 137.50 137.50 137.50 140 00	2 3.00 2 3.00 2 3.00 2 3.00 2 3.00 2 3.00 2 3.00	0 5 5 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	0 70 , 0 73 0 74 0.63 0 64	10 0 (3 10 0 63 10 0 63 5 0 31 0 0 63 5 0 31	10 0 1 2 0 0 6 0 0 12 0 1	- 15		-	1 0 3 1 0 3 1 0 3 1 0 3 2 0 6 1 0 3 1 0 3		-	- 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	7 30 7.30 7.30 7.30 7.30 7.30 7.30	3.30 2 3.30 2 3.30 2 3.30 2 3.30 2	4.80 ° 4.	4.20 2 4.70 2 4.70 2 4.70 2 4.20 1 1.20 1	4.80 4.80	2.80 13.44 13.44 13.44	3' 12 29 5' 16.07 3' 10 40 5' 9.14 2" 9.14	16 0 22	8 O 34	2 0 13 - 4 0.26 - 2 0.13 2 0.13	1 0.07 -		223-70 210 (/8 230 26 224.11 221.58 206.74 1(/7 11	0 00 0.71 1.00 0.00 0 07 0 00 0 00		-	37 37 27 31 28 20	Began sealing caisson at 9:4

### TIME, COST AND MATERIALS USED IN FOUNDATIONS. PIER II.

nie pril	Vg t	on Fr	incr	Lank	10	fr	Vi	0	( 4	<b>S</b> 11	16.4	7111	اديد	×-5-	15 -17	200 4	y se l	-1	h. i	n <sub>k</sub>	. r	r	1 1500.5	, VI	1	in the	ſ	٠.	> 21	P	s 1	A	W 3 - c	4, 1		, , !	16	fn e i	L. Ach	or 1	a sps	Remarks
Amount	Days Amo.nt	D11.	Amount	My out	DSA	A 0.0	_	1 · 1	17	*	1 1		-	, m,	-	<u> </u>	£		ž e	î	D 5	1 Pr	4.0.0	Š	4	Inys	- 1 - 1	16- 3	2 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Pate	, a	. ,	yw. unt	7 W				11,	Care N	r (d	No of	
1 47 1 47 1 47 4 47	-	o,	1 63		3, 31,	54 17 +; 45 ; 00							40	' 4+			16	, on		-		ŧ								61 11	D 14 C 3 <sub>4</sub> v 14				62	71 9 71 9	56 6 6					Capenters and laborers began lowering cais
6 00	1 3 3	57 57 3 7	1 50 , 00 , 10	3 , 50	5 5	° - 1	,	· ))	3 3	. )	;`.	13 1	3	, לי ל יוני	0.5	-	, ,	3 X 3 X			3 30 1	4 4	: 2	to 1	4 80 2 40		7	1+ 35 15 0	8 0 4	*	0 12 8	, 57	0 7	- - 0 T	17 33		5-14 5-10 5-10 66 5-02	and 10	0 e	3 <sup>2</sup>	20	Worsing in 1 III
6 00 6 00 6 00	1 3 5	50 2 50 3 51 3	1 50 6 00 , 00 3 00	1 2 2 1 2 7 1 2 2 1 2 2		10 ,5		\$0 3 H 3 H 1 m	; / ; ; ;		v I	; +	3	3.17	9 3	2	۵,	3 6	7 ,	1		107	; 1 ;	2	2 t2 1 t 2 t 2 t 2 t 2 t 3 t 3 t 4 t 3 t 4 t 4 t 5		e C	23 40 11 7e 13 40 23 40	n 13	* * * * * * * * * * * * * * * * * * * *	0.12 6 5.15 0.12 0.0	n 10 , 50	1 00		107 145	75 73 60 30, 0	b 1 1 2 2 3 3 1 1 2 2 3 3 1 1 2 3 3 3 1 1 2 3 3 3 1 1 2 3 3 1 1 2 3 3 1 1 1 1	n se inda	¥.,	*n 35	23 33 25	Working in P III
				<i>y</i>								4								-					-							-		-			-		-	- ' -	-	Work suspended. River breaking up
3 00 6 00 6 00 6 00 6 00 6 00 6 00 6 00	1 3 1 3 1 3 1 3 1 3 3 3 1 3 3 1 3 3 3 3 1 3	22 1 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1	3 10 000 3 00 0 00 0 00 1 3 7 1 3 7	3 1 5 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	0 16	24 73 4 73 24 13 47 13 75 77 10 75 10 75		* 1		+ + + + + + + + + + + + + + + + + + +	77 77 77 77 77 77 77 77 77 77 77 77 77	; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ;	7 3	o n !,	+ * * * * * * * * * * * * * * * * * * *	3	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	of n y	33		4 10 2 1 10 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 100 + 200	2 4		5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5		6 6	· · · · · · · · · · · · · · · · · · ·	1 3 3	14 5 7 7 7 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0 1 1 0	u 40 u 40 u 40	, o" 1 - 7		5 5 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	37 24 0 5'	3 20 1 2 1 2 1 2 2 2 2 2 2 2 2 2 2 2 2 2	and	4	## + 44 AM 3) 77 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	3+ 77 73 73 73 74 77 76	testing to describe a count of algono-
6 00 6 00 6 00 5 00 6 00 7 00 6 00 7 00 6 00		33 33 33 34 34 4 33 34 4 33 34 4 33 34 4 33 34 4 33 34 4 33 34 4 33 34 4 34 3	h 50 13 Ju 3 Ju 3 Ju 3 Ju 3 Ju 3 Ju 3 Ju 3 Ju	2 5 0 2 5 0 3	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	33 m 11 <sub>1</sub> 3 4 1 <sub>2</sub> 3 5 2 1 3 115 3 6 1 3 0 1 0 0 1 0 1		19 No. 19			3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	3	+ 2			* *	32		3 - 6x		1 1 7 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	*** * * * * * * * * * * * * * * * * *	2 4:	10 1 11 1	4 10			17 1.	*	i		2.15	0.00	^ (1	127	4	73 1 30 J		31.	30	514777	Types managementing Working in Table

### APPENDIX E. Continued.

### PIER II.—CONTINUED.

5 0 0 3 33 3 3 3 3 5 6 6 6 6 6 6 6 6 6 6 6 6	# 5 # 4 # 5 # 5 # 5 # 5 # 5 # 5 # 5 # 5	1 00.50 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	47.7477 47.747 47.747 47.747 47.747 47.747 47.747 47.747 47.747 47.747 47.747 47.747 47.747 47.747 47.	Another Age 20 5 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Amount Amount The Control of the Con	Am, upri	Amount Amount	3 1.02 3 1.02 3 1.02 1 0.34	Pino Pino Pino Pino Pino Pino Pino Pino	Dys. 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2	Andrew Control of the	4.80 1 4.	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	7.0000 4.000	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	11.35 11.35 10.90 17.00 15.68 17.00	sund omy 8 0.34	8 0 12 8 0.12 8 0.13 8 0.13 8 0.13 8 0.13 8 0.13 8 0.13	In the state of th	1 0 07 1 0 07	Amount	145.87 (88.87) (190.71) 194.70 193.43 193.43 193.23 197.23 197.23 200.25	0.02 c 0.02 l 19 5 5 4 10 17 0.14 17 0.14	Her T	24 28 23 27 26 21 22 22 24 11 13 15	Working in Pier III.  Working in Pier III.  """  """  """  """  """  """  """
6 00 3 33 3 4 3 7 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	9 75 3 3 4 4 1 1 1 1 2 1 3 2 1	L L L 105 105 105 105 105 105 105 105 105 105	2 3 25 4 1 00 2 3 100	3 0 70 4 8 6 6 7 1 8 6 6 7 1 7 8 6 6 7 1 7 8 6 6 7 1 7 8 6 7 1 7 7 8 6 7 1 7 7 8 6 7 1 7 7 8 6 7 1 7 7 8 6 7 1 7 7 8 6 7 1 7 7 8 6 7 1 7 7 8 6 7 1	- 0 76 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	5 0 06 4 5 0 07 6 0 07 6 0 07 7 0 03 1 0 02 7 0 03 1 0 04 1 0 04 1 0 04 1 0 04		3 3 3 1	3 1.02 3 02 3 1 03 1 0.34		2 7.60 2 7.60	3 · 30 · 3 1 3 · 30 · 3	4.80 3 4.80 3 4.80 3 4.80 3 4.80 3 4.80 3	4.20 1 4.20 2 4.20 3 4.20 3 4.20 3 4.20 2 4.20 1 4.20 1 4.20 1 4.20 1	4.80 4.80 4.80 4.80 4.80 4.80 2.40 2.40 2.40 4.80	9° 16.32 4° 16.32 4° 9° 16.32 4° 9° 16.32 4° 9° 16.32 4° 16.32 4° 16.32 4° 16.32 4° 16.32 4° 16.32 4° 16.32 4° 16.32 4° 16.32 4° 16.32 4° 16.32 4° 16.32 4° 16.32 4° 16.32 4° 16.32 4° 16.32 16.32 4° 16.	11.35 10.90 17.00 15.98 17.00 15.98 17.00 16.60 17.00	8 0.34	8 0 12 8 0.12 8 0.12 8 0.12 8 0.12 8 0.12 8 0.12 8 0.12 8 0.12 8 0.13	-	1 0 07 1 0 07 2 0 07 1 0 07 1 0 07 1 0 07 1 0 07	-	192.93 189.53 189.53 193.43 192.23 197.22 200.25	0.02 c 0.02 c 0.02 c 1 19 54 0.10 1.30 c 44 1 1 1 0.14 1 0.92	ay	24 28 23 27 27 26 21 22 22 14 13 15	Working in Pier III.
0 000 3.323 4 33.0 0 001 3.73 1 13.0 0 00 1 3.73 6 10.3 0 00 1	13,00 ± 5 00 13,00 ± 5 00 13,00 ± 5 00 13,00 ± 5 00 13,00 ± 5 00 19,50 ± 5 00 19	0 40 100 00 107 75 00 4 107 55 107 50 107 55 107 50 107 55 107 50	1 00 1 100 1 100 2 100 2 100 2 100 1 100 1 100 1 100	3 0 71 6 4 0 94 6 3 0 70 6 4 0 98 4 5 5 1 28 10 6 1 53 6 1 1 73 10 1 1 73 10	038 3	0 03 0 04 0 10 0 10 1 0 04 1 0 04 1 0 07	-	3 1	3 1 03 3 1 03 1 0.34		2 7 60 1 7 60 2 7 60 2 7 60 2 7 60 1 7 60 2 7 60 2 7 60 2 7 60	I 3.30 1 I 3.30 1 I 3.30 1 I 3.30 1 I 3.30 1	4.80 2 4.80 2 4.80 3 4.80 3	4 20 7 1,10 7 4 20 2 4 70 4 4 20 1 4 20 1 4 30 1	4.80 2.40 2.40 2.40 4.80	g" 10 32 5, g" 10 25 4 11 19.74 5, 12 21.72 4" 12 21.06 5	15.98 17.00 15.98 17.00 16.60	 8 0.34	8 0.12 8 0.13 8 0.13		1 0 07 1,0 7 1 3 7 1 0 07 1 0 07		189.53 193.43 192.23 197.22 200.25	1.30 0.44 0.17 0.14 0.92		21 22 22 14 13 10	
00 , 120 ( 1 )	19 90 0 9 10 10 10 10 10 10 10 10 10 10 10 10 10	10 13 13 13 13 13 13 13 13 13 13 13 13 13	2 3 4 5 5 7 5 7 5 7 5 7 5 7 5 7 5 7 5 7 5 7	7 178 8 5 137 6 + 102 6 + 02 6 + 153 8 5 136 5 1 102 N	0 50 8 3 3 0 50 50 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	5 0 J 3 0 04 - 5 0 00 - 5 0 00 - 6 0 J 6 0 J 7 + 8 0 00 - 9 0 00 -	0 2, 7	005 1	1 0 34	1 0 10 1 0 00	2 7.60 2 7.60 1 7.60 2 7.60	3.30 2 3.30 2 1 3.30 2 1 3.00 2	4 80 2 4 80 3 4	4 70 2 4	1 %0 1 %0 1 %0 1 %0 1 %0 1 %0 1 %0 1 %0	6 32 5 5 5 6 16 16 25 5 5 6 16 16 25 5 5 6 16 16 25 5 5 6 16 16 16 16 16 16 16 16 16 16 16 16 1	15.64 17.35 15.98 15.98 15.98 10.66 17.00 13.66 17.00 16.66 17.00 16.66 17.00 16.66 17.00	8 0.34 8 0.34 8 0.34 6 0.34	8 0.11 8 0.11 5 0.10 8 0.11 8 0.11 8 0.11 8 0.10 8		1 0.07 1 0.07	01.	248.51 2.5 33 254.42 252.38 257.19 256.34	0 40 0 35 0 004 1 53 0 11 1 65 0 11 1 65 0 001 1 75 0 13 0 13 0 13 0 13 0 13 0 15 0 17 1 5 1 5 1 5 1 5 1 5 1 5 1 5 1 7 1 7 1 7 1 7 1 7 1 7 1 7 1 7	s	23 225 31 21 21 26 32 28 28 28 28 22 22 24 22 24 22 27 29 29 22 25 27 29 28 27 27 28 27 27 28 28 28 28 28 28 28 28 28 28 28 28 28	
6 00   3 33   16 1 6 1 6 1 6 1 7 1 6 1 6 1 7 1 7 1 6 1 7 1 7	16 15 5 5 500 17 30 2 1 100 17 30 2 1 100 17 25 2 5 50 17 52 7 5 50 19 50 7 5 50 10 50 7 5 50 10 50 7 5 50	0 54 13 00 15 15 15 15 15 15 15 15 15 15 15 15 15	2 3 25 25 25 25 25 25 25 25 25 25 25 25 25	1 10 0 1 10 0 1 130 6 1 137 6 1 138 8 1 138 8 1 1 10 0 1 1	0 35 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	3 0 04 8 0 10 3 0 16 7 0 31 3 0 04 8 8 0 10 10 11			1.30		2 7.60 3 7.60 3 7.60 3 7.60 3 7.60 3 7.60 2 7.60 2 7.60 3 7.60 3 7.60 3 7.60 2 7.60	1 3,30 1 1 3,30 1	4.80 2 4.80 3 4.80 3 5 4.80 3 5 4.80 3 5 5 6 6 7 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	4 70 12 4 20 1 1 30 1 4 70 2 4 70 2 4 20 2 1 10 1 4 30 1 4 30 1 4 30 1 4 30 1 4 30 1	4 80 4 70 1 4 80 1 4 80 1 4 80 2 40 1 40 2 40 1 40 1 40 1 40 1 40 1 4	141 24.48 44 44 54 44 24.48 54 44 24.48 54 44 24.48 54 44 24.48 54 44 24.48 54 44 24.48 54 24	7,00 17,00 17,00 15,64 17,00 17,00 17,00 17,00 17,00 17,00 17,00 17,00 17,00	8 0.34 8 0.34 -	8 0.17 8 0.15 8 0.15 8 0.13 8 0.13 8 0.13 8 0.17 8 0.17 8 0.12 8 0.13 8 0.13 8 0.13 8 0.13 8 0.13 8 0.13 8 0.15 8	_	I 0.07 1 0 07 I 0 07	_	260 19 25- 47 1, 239 96 239 96 237 93 237, 93 233, 40 222, 45 1-7 50 205, 24 211 38 115, 21 117 99 111 64	0 07 0 16 0 16 0 30 0 17 0 02	a),	333 334 335 26 27 26 27 27 25 25 27 25 27 25 27 25 27 25 27 27 25 27 27 27 27 27 27 27 27 27 27 27 27 27	Begun serang causson.  Took off an at 705 a.m.

### TIME, COST AND MATERIALS USED IN FOUNDATIONS. PIER III.

rincipal reneta l	Night Fremin,	Sub	Tenrie	k F	ressure Men	Coffeeho	use Ce	stee	Sugar	Car	idles.	Red	Wi	iek	Coul for Broting	Linses	d End	zineer I	Nig's Pudheer	Pireme	n C	ond sers	Pump Men	La sorers Hoss	on C	Coal for	Signal Ou	Black Oil	Tallow	Waste	Cont Oil	Totals for Euch Day	Feet Sunk	N.a	Water P. mps	A	Air J	
Amount,	Days Amount.	Days	Travs	An ount.	Amo. m	Livs	T.	A * Oun'	1.5.4 N-16.0	No.	Ато пс	Lbs. Amo nt	sq I				1					tino "					2	Pints Arount	**	Lt+4 Amount	Pints Ar nunt		p. eur,		Hrs No			Remarks.
6.00	1, 3-33 0 2.67 1 3-33	2 6.or	D -	47°	91 87 90 27 (7 5-	-	- 15	~		1.	-	to to	-	= 1		16 1.	000	-	-	\     -			_	-	-   -			16 0 °C 16 0 20 16 0 20	-			110.80 135 14 80.03	4 70 5.80 5 60 0 28 0 32 0.00 0 00				-  1 -	owering caisson. Jegan concreting in a. m.
6.00 6.00 6.00 6.00 6.00 6.00 6.00 6.00	1 3 23 1 3 23	1 3.0	0 7 4 0 0 2 4 0 0 2 4 0 0 0 2 4 0 0 0 0 0 0	50 33 50 0 50 25 50 25 50 16 50 30 1.50 30 1.50 30 1.50 30	67.50 67.50 36 00 13 50 70 15 11.25		The water with the comment of the co	9.534.53344.67548675334334480.00.00.00.00.00.00.00.00.00.00.00.00.0	6 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	6 3 3 4 4 6 7 3 1 1 3 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4 0 2	<del>-</del> -	0.05	1.36 1.36 1.36 1.36 1.36 1.36 1.36 1.36	2 0	2	8 15 7 30 7 30 7 30 7 30 7 30 8 95 1 7 30 7 30 7 30 7 30 7 30 7 30 7 30 7 30	3.30 3.30 3.30 3.30 3.455 4.55 3.30 3.30 3.30 3.30 3.30 3.30 3.30 3	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4		# 4 20 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4 + 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8		7 6 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	37.74 37.74 37.36 1.7.36 2.0.35 19.38 20.74 19.00 20.74 19.00 20.74 19.36 20.74 19.36 20.74 19.36 20.74 19.36 20.74 20.74 20.74 20.74 20.74 20.74 20.74 20.74 20.74	8 0 34	8 0 10	3 6 6 50			30 75 8 30 83-1,7 88-50 54-86	9 80 309 3.15 30 48 8 0 07 3 33 6 30 0 0.00 1 0 45 1 97 3 0 0 0 0 0 1 3 1 7 1 7 1 7 1 7 1 7 1 7 1 7 1 7 1 7	563 % A	2 12 3 3 4 4 3 3 3 5 6 4 3 3 3 5 6 4 3 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	8 1 1 1 5 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	16 13 18 16 16 17 17 17 17 18 18 18 18 18 18 18 18 18 18 18 18 18	
1	1 3 23 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	1 4.8 7.7 4 13.0 4 13.0 4 13.0 4 13.0 4 13.0 4 13.0 6 19.9 9 19.9 9 19.9	85 2 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	13   15   15   15   15   15   15   15	33.75 30.00 31.75 30.00 31.75 30.00 31.1.25 30.8.75 108.75	6. 医不存在 医医医医 医自己 医自己 医自己 医自己 医医医氏试验 医二种	25 1 2 2 3 4 3 2 2 4 + 1 0 3 5 5 7 4 7 3 4 7 4 7 5 5 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0.30 0.40 0.40 0.80	+ 0 2 2 3 3 4 4 0 2 3 3 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	4 3 4 4 6 6 8 2 10 10 1 3 2 6 6 6 6 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 0.13	1 1/4	0 0,	1 1.36 1 1.36	1 0	04 U 2 2 3 2 2 3 3 2 2 3 3 2 2 3 3 3 2 2 3	7-30   7-	3 300 3 300 1	2 2 4 4 7 7 7 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	0 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	4 no 2	4.880	17 20 .1 20 .7 20 .10 15	40 6° 40 6° 40 6° 36 5°	20.74 19 04 20.40 19 33	0 34	8 0 10 10 8 0 10 8 0 10 8 0 10 8 0 10 8 0 10 8 0 10 8 0 10 8 0 10 8 0 10		1 0 0 0   1 0 0 0 0 0 0 0 0 0 0 0 0 0	0.11	204-24 201-27 1)2-73 130-51 150-51 150-51 150-51 150-51 164-30 184-30 255-7 245-30 255-7 245-30 255-7 245-30 255-7 245-30 255-7 245-30 255-7 245-30 255-7 245-30 255-7 245-30 255-7 245-30 255-7 245-30 255-7 245-30 255-7 245-30 255-7 24	0 03 0 03 0 03 0 03 0 03 0 03 0 03 0 03		2' at 4 6 , 3 3 . a 3 3 00% 3	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	21	ump men running clay boat.
6.00 6.00 6.00 6.00 6.00 6.00 6.00 6.00	I 3.57 I 3.57 I 3.57 I 3.57 I 3.57 I 3.57 I 3.57 I 3.57 I 3.57 I 3.57	\$\frac{17.8}{4}\$ 17.8 \frac{1}{4}\$ 13.0 \tilde{0}\$ 1.95 \tilde{0}\$ 15.5 \tilde{0}\$ 15.5	8 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	00 62 <sup>5</sup> 00 61 <sup>6</sup> 00 62 <sup>5</sup> 00 62 00 62 00 58 00 58 00 60 00 58	156 25 153 75 153 75 153 75 153 75 155 00 145 00 157 30 147 25 151 25 148.75	2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	00 2 00 5 00 7 00 7 00 5 00 4 00 5 00 5	1 13 1 13 6.67 1 13 0 90 13 1 13 C 90 1 35	10 0 6 10 0 6 6 0 3 10 0 6 10 0 6 5 0 3 .0 0 6 0 .0 10 0 6	2 3 3 3 4 2	0.03 0.05 0.05 0.03 0.03 0.04 0.01	2 0	- - 8 & ,	o 05	3 1 02 4 1.36 4 1 36 3 1 02 3 1 02 2 0.66	3 0	O4 2	7 30 7 30 7 30 7 30 7 30 7 30 7 30 7 30	7 3.30 7 3.30 7 3.30 1 3.30 1 3.30 1 3.30 1 3.30	3 4.8 3 4.8 4.8 4.8 4.8 2 4.8 2 4.8 2 4.8 3 4.8	0 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	4 10 2 1 10 1 1 20 2 1 10 2 4 20 3 4 20 3	4.80 4.80 4.80 4.80 4.80 4.80 4.80 4.80	9 It (18, 12 20, 12 20, 13 12 20, 13 12 12 20, 13 12 20, 13 12 20, 13 12 20, 14 13 12 20, 15 15 15 15 15 15 15 15 15 15 15 15 15	36 5 5 5 6 6 5 5 6 6 5 6 6 6 6 6 6 6 6 6	19:72 20:74 	8 0.34 5 0 3n - 5 0.34	S 0 12 S 0 2 S 0 2 S 0 2 S 0 2 S 0 2 S 0 2	-	. 000	16 0 20	249.74 257.57 258.37	0 0( 0 32 2 15 7 09 3 12 0 1, 1 35 3 30 1 16	41		2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	23 23 23 23 23 23 24 24	

### APPENDIX E. Continued.

PIER III. CONTINCET

The content of the	rincija. Nighi ore i sii Pure in	nt inn Fo	bus on on	Look Fengers	Pi	ress re Men	Cof hot Me	Tee use m	Coffee.	Sugar	Can .	les k.	G Le 1d	Wick	s oct for he terns	[1]	ı I n	Day ighter a 1	Vigh.	L.eir	t. Mari	~0**	At mits	h leave	P	Mar.	gnal Oil	Pink D	Tillow	N was	conf C	T Cds	or s	Mate	Pu	er ps	Punips-	, CEMARKS
Second   Column   C	Amount Days,	Days	Amo-int	Amount.	Days	Amount	Days	Amount.	Amount	I bz. Amount	No	Antount	Anount	An sunt	Bex %	Pars.	Days	Amount	Атови	Thys	, Days	Amaunt	Amount	Miscant	I ny	A. JIII.	Ar wink	Pats Ansant	Ths	- Lbs Amount	Prits		1		Hours run	No. of	No of revolu- tions.	
## Section   1	6 co 1 3.5 6 co 1 3.5 6 co 1 3.5 6 co 1 3.5	5   5	16 35	2 5 00	57	76.10	3	3.00 4 3.00 5 3.00 4 3.00 5	1 13 0.90 1 12	5 0.31 5 0.31 10 0 65	6 0	03 2	0.12	0,	2 0 0	5 52 0.0	2, 3, 2, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	7.30 1 7.30 1 7.30 1 7.30 1 7.30 1	3.30 3.30 3.30 3.30	2 48 2 48 2 48 2 48 2 48 1 18	0 2 3	4 30 3 4.20 1 <sup>2</sup> 4.30 7 4 34 2 4 30 2	1 ho 1 ho 1 ho 1 ho 1 ho 1 ho	12 20.40 15' 6 3' 12 '10.40 12 '20.41 12 '50.40	0 6 6 0 5 5 0 0 1 0 6 6	20 00	5	δ 0 12 δ 0 3 δ 0 3 δ 0 2		1 0 07		25° 4 148.7 236 3 139 8	5 D.I	0 gravel			25	Wesking in Per U
6.00   1 9-79   1 9-00   1 9-0	- 1 3.5	20					1	- 3	0.08	5 0 32	-	0.2	_		+ 17	5	-	-01		1 2 4		2 .0 1	2 .0.1		5 61	20 74 15 38 4 60 4.60 - 20 74 8	0.34	8 0.12 8 0.12	-	1 0 0,		150 9 16 7 4 6	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	06 · · · · · · · · · · · · · · · · · · ·		-	24 - 23 26	Week ng in Paer II
0 00 1 313 4 446 1 239 84 1129 2 100 7 000 0 001 2 000 0 001 10 0 0 0 0 0 0 0				1	1		1 1						-	-1	2 0 6 3 1.0					1 4 6 3 4 8	0 7	4.20   2	4.80 4.80 4.80	13 20.4 13 20.4	0 6	22 78	0 34	8 0.12	-	1 0 0		352.)	+ 0	59 36 24 clay				
1 0 0 1 2 3 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1	6.00 1 3.2	23 6	19.50 —	1 5,00	66*	-			0 68 0 23 -	5 0.3:	7 0		-	-	4 T-3 - '- -	- 1	31	7.3° 1 7.3° 1	4.12	- =	-	1.37 2 4 20		12 <sup>1</sup> 20 9	7 6° 6′	22 78 -		8 0.12	-	-		273 9 43.0		01 "	_       	-	21	Work suspended River blea
6 co   1   3   6   6   7   7   1   1   1   2   0   2   1   1   1   1   1   1   1   1   1	6 00   3 2 6 00   3 2 6 00   3 2	23 6 23 6 23 4	,4 03	1 7.50	3 44	111.55	2	3.00	0 67 0 68 1.12	6 0 39 4 0 2- 5 0 32 7 0 50	3 0	0 13 -	-	-			- 1	7 30 1 7 30 7 30 1	3 30	4 4 4 4	6 3 3	4.20 2 4.20 2 4.20 2	1 00	2 3 4	0 6	22 44	5 0.1.	\$ 0 12 8, 3 1° 4 0 06	-	. 00 0 Y 1 0.7	, -	254 8 337 3 300.7 61 0	3		-	440	33 23 70 13	
6 00 1 1 3 3 6 1,50 2 5 00 5 1 1 3 0 0 1,50 2 5 00 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 5 1 1 3 0 0 1,50 2 5 0 0 1 1 3 0 0 1 1 3 0 1 1 1 1 1 1 1 1 1	6.00 t 3 2 9 3.00 l 3 2	23 6 23 1	15,30	3.79	5 29	72 50	B .		1 13	7 0 3	_			_			1	7 30 1	3 30	2 4 8	0 2 10 -	4 70 3	-	5° 4.5	, 6	=		4 0 00			: =	131 2	is .	_	_		33	Working in Pier II
6.00   133   6   153   6   153   1   150   1		-  -		_ =	-	-	-				-		1 1 1							-		-				***		- =	  -  -		= -		_	-	-		-	
6 00   3   4   5   5   7   5   7   5   7   5   7   7		-	-		-	_		-  -	-		= = = = = = = = = = = = = = = = = = = =		1								-		-			-				- 1 - 1	-		-	-	_	1 1 1	-	
o o t loss off an at 8 oc a m	6 co   1 3 3 3 3 6	334 6 34 6 33 6 33 6 33 6	19.50 19.50 19.50 19.50 19.50 19.50	2 5 00 2 5 00 2 5 00 7 5 00 2 5 00 2 5 00	58 50 55 55 55 55 54	145 JO 151-25 147 50 138-75 37 50 136 25	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	3 00 2 3 00 3 3 00 3 3 00 3 3 00 3 3 00 3 3 00 3 3 00 3	0 45 0 90 0 65 0 65 0 60 0 60	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	30 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.15	- 1	-	3 10	4 r	12 3 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	7.60	1 3.30	2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	00 2 2 3 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	+ 20   1 + 70   2 4 70   2 4 70   2 4 20   7 + 20   2 + 70   3 + 20   1 4 20   4 + 20   4 + 20   7 + 20	2.40	5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	11 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	22 76 21 76 22 76 22 76 22 76 21 76 21 76 21 76 21 42 32 78 31 78	8 0 34	8 0 12 8 0 13 8 0 13		1 0 0	7 7 8 0 7 7 7 8 0 7 7 7 7 7 7 7 7 7 7 7	236 : 140 . 140 . 246 . 242 . 233 . 233 . 233 . 233 . 234 . 25 . 25 . 25 . 25 . 25 . 25 . 25 . 2	7 0 0 17 0 0 17 0 0 16 0 0 17 0 0 17 0 0 17 0 0 17 0 0 17 0 0 17 0 0 17 0 0 17 0 0 17 0 0 17 0 0 17 0 0 17 0 0 0 17 0 0 0 0	01 00 13 52 38		-	23 24 22 76	
655.80 357.51 14.6.8; 505.25 11329.01 3°0.75 80 11 41.50 5 94   4 56 0 60 51.00 1.86 788.48 350.6° 513.40 447.13 4°4.80 1100 41 23-3-23 9.5° 11 97 6.29 7 56 0.99 20015 12					-		,~					1					86		-				424 80	1	- ,							11	1	0.1				look off an at b ,c a m

### APPENDIX E.-Continued.

## TIME, COST AND MATERIALS USED IN FOUNDATIONS. PIER IV.

Date	Principal   Poraman	Night Foreman	Suh Foremen	lock fer	Pressure Mun	Coffee house Men	Coffee	Sugar	Сппа,е	s Red Le	ad Wask.	Coul for heating, 200 s	Lanseed	Day Engineers	Night Engineer F	Tromen	Coal Passers	Lab Ven	Laborers on	Coal for Howers,	Signal Oil	Black Or	Tallow	Waste.	Cont Oil	Totals for	Feet Subspir	Mate 1	Cameron Pumps	Air Pumps	
1886 —	Amount	Amount	Agrount,	Drys Ario-at	Days Amount	Da, s	Libs	IV s.	No	1 hs	Libs Atroons	Jox+3	Pan's.	Баля	Days	Ano s	At our t	An. of	Do c	Ten.	Amo at	Fars.	Lts. Amount	Amount ,	Perts		cary	- 1	Hours No of run revolu- tions.		Remarks,
Aug 22 21 24 25 26 27 28 29 30 31 Sept	1 6.00 1 6.00 1 6.00 1 6.00	1 3-23	3 9.00	2 4.50	32 <sup>b</sup> 73 12 32 77 00 29 65.75 34 76.50 34 76.50 34 76.50 34 76.50 34 76.50 35 8 .00 31 70 37	3,2	15 4 05	5 5 0 3	31 6 0 0 1 31 3 0 0 1 30 8 0 0 1	08 .		2 0.63 2 0.63 3 0.63	-	7 7.30 2 7.30 3 7.30 3 7.30 3 7.30 2 7.30 2 7.30 2 7.30 2 7.30	3 30 2	4.80 4.80 4.80	1 4 10 2 4 20 3 4 30 3 4 30 3 4 30	4.80		3' 11.90 3 to 20 6' 22.40 7 23.80 7 23.80 7 24.82 7 23.80 7 23.80 7 23.80 7 23.80	8 0 34	8 0.11 8 0 1 6 0 11	.0 0.65 - 10 0.63	1 0.07	-1 -	130 87 134 33 13, 140 63 -52 26 153 96 153-95 151 07 -57 29 148-23	1 45 1 77 0 17 3 31 0 32	14 2	32 33 33 37 30 30 40 30 40 30 40 40 40 40 40 40 40 40 40 40 40 40 40	30 24 23 20 15 16 13 14 10	Started air pumps at 7 o5 a.m. Sta ted Cameron pumps at 7 30 a.m.
1 4 5 6 7 8 9 10 11 13 14 11 12 12 12 12 12 12 12 12 12 12 12 12	1 0.00	1 3 3 4 1 3 3 3 4 1 3 3 3 4 1 3 3 3 4 1 3 3 3 4 1 3 3 3 4 1 3 3 3 4 1 3 3 3 4 1 3 3 3 3	2	\$ 4.50 2 1.47 3 1.50 2 1.50 1 1.50 1 1.50 2 1.50 1 1.50 2	\$\frac{1}{2}\$  \frac{1}{2}\$	2	4 4 3 3 5 7 4 5 5 7 7 7 4 4 7 9 7 7 7 7 4 4 7 9 7 7 7 7 7	4 0 2 5 6 6 6 8 0 5 7 6 6 6 6 8 0 6 7 6 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7	13 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	104	1 4 0.05	3 0.94 3 0.95 1 0.31 3 0.95 3 0.94 3 0.95	3 0.13 	2 7 30 2 7 30 2 1 30 2 1 30 2 1 30 2 1 30 2 7 30	1 3:30 2 1 1 3:30 2 1	4,80 1,80 1,80 1,80 1,80 1,80 1,80 1,80 1	2 4.00 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 4.30 3 4 20 2 2 4 20 2 2 2 2	4.80 4.80 4.80 4.80 4.80 4.80 4.80 4.80		777777779658 157876	8 0.34 8 0.4 8 0.4 8 0.34 8 0.34	8 0 11 8 0 11 8 0 .0 6 0 10	10 0.64 10 0.64 10 0.63 10 0.63 10 0.63	1 0.07 1 0.07 1 0.07 1 0.07 1 0.07	5 0 11	172.00 183.85 .53.40 180.63 186.07 181.00 180.41 168.59 182.69 184.53 179.46 181.12 187.04 186.55 183.55 183.55	0 02 0.00 7.2y 1.75 2.76 1 0y 0 20 0 3h 0 41	3 3 3 3 3 4 3 3 4 4 3 4 4 4 4 4 4 4 4 4	4 4 3 39 39 39 31 31 31 31 31 31 31 31 31 31 31 31 31	16 15 14 10 21 21 20 14 21 22 24 25 27 32 27 32 32 31 32 31 32 31 32 31 32 31 32 31 32 31 32 32 32 32 32 32 32 32 32 32 32 32 32	Pump men running clay hoist.
Oct. 1 2 3 3 4 5 6 7 8 9 9 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 6.00 1 6 00 1	1 3 - 23   1 3 - 23   1 3   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3 - 23   1 3   1 3 - 23   1 3	19.50	3 5.00 1 5.00 2 5.00	56 141.43 556 140.75 566 140.75 14	2 3.00 2	10	8 8 8 8 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	10	13	0 5 0 10	3 0.95 3 0.95 3 0.95 3 0.94 3 0.95	3 0 19	7,30 7,73 7,73 2,7,	9,30 - 1	4.80 2 4.	4   -20   1   -4   -20   1   -4   -20   -2   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -4   -20   -20   -4   -20   -20   -4   -20   -	4.80 4.80 4.80 4.80 4.80 4.80 4.80 4.80	12 ( 80 ) 1 " 6 66  8 .40  2 1 ( 16.38 ) 11 16.50  12 16.80  12 16.80  12 16.80  12 16.80  13 17 10.84  15 12 18.43  15 14.43  15 14.43  15 14.43  15 14.43  15 14.43  16 14.43  17 16.43  17 16.43  18 17 16.43  19 16.43  10 16.	46 17 76 18 76 17 77 17 18 18 18 18 18 18 18 18 18 18 18 18 18	16 0.66 8 0.34 5 0.34 6 0.34 8 0.34 8 0.34	8 0 10 8 0 10 10 10 10 10 10 10 10 10 10 10 10 1		1 0.07 1 0.07		237-33 244-42 232-66 228-36 229-70 229-70 223-39 221-44 219-69 221-69 176-99 156-99 156-99 169-10 239-87 216-08 226-38 235-79	1.33	Acel -		25 336 34 377 27 25 25 25 27 27 27 27 27 27 27 27 27 27 27 27 27	
3 4 5 6 7 8 9	6 00 II 6 00 II 6 00 II 6 00 II 6 00 II 6 00 II 6 00 II	3-33 6 3-33 5 3-34 5 3-33 4 3-33 4 3-33 6 3-34 3	19.50 17.88 16.25 1,.50 14.63 19.50 10.25 19.30 9.75	2 ,00	56 140.00 53 133 75 54 136.25 54 136.25 56 14 1.25 56 141.25 57 131.25 13 32.50	3 3 00	0 4 0.67 0 6 00 0 6 100 0 5 0 64 0 5 0 64 0 6 100	8 051	5 00	6 -	-	3 0.94 - 3 0.95 3 0.95 -		2 7-30 2 7-30 2 7-30 2 7-30 2 7-30 2 7-30 2 7-30 1 3-30 2 7-30 2 7-30 2 7-30	I 3 30 7 I 3 30 7 I 3 30 2 I 3 30 7 I 3 30 7	4.80 2 4.80 3 4.80 3 4.80 3 4.80 4 4.80 4	1.30 3	4.80 4.80 4.80	10 <sup>9</sup> 14.28 8° 17.76 9° +3.44 6° 11.8 3° +46	4' 13-14 5' 13-26 3 12-32 2' 9-18 0' 72	8 0 34	8 0.10 8 0.10 8 0 10 8 0 10 8 0.10 8 0 10 8 0 10 8 0 10	- 1 - 1	0.07 0.07 0.07 0.07 0.07 0.07 0.07	.   	235 23 214 53 213 44 107 24	0.64 40 001 40 001 40 001 40 001 40 001 40 001		-	29 24	Began blling caisson at 11:25 a.m. Took off air at 7:20 a.m.
	480 00	262.30	1177.37	388.75	9018.13	246.00	0 50.57	30.04	5.8	5 4-4	0.57	30.87	2.00	580.00	264 00 3	381.60	331.80	348.00	×24.88	1102.84	8.84	8.13	6.36	5-53	0.22	5559.06					
																					Otl	her expense	s charged t	o sınking	5	1595-93	\$17154.99				

### APPENDIX F.

#### SPECIFICATIONS FOR SUPERSTRUCTURE

#### GENERAL DESCRIPTION

The superstructure will consist of three main through spans and six deck spans three of which will be at each end of the structure.

Each through span will be 375 feet long between centers of end puss, divided into fifteen panels of 25 feet seach. The trusses will be 50 feet deep and placed 12 feet apart between centers. The top chord, end posts, bolsters, rollen, becamp plates, purs and all eye-bars except counters and vert.cml asspencers, will be of steel- all other parts will be of wrought iron except the wall plate pedestals and ornamenta, work, which will be cast. Each span will contain approximately 486,000 pounds of steel, 484,000 pounds of wrought tron and 21,000 pounds of cast ron

The total estimated weight of the entire structure is approximately 4,000,000 pounds.

#### PLAN5

Full detail plans, showing all dimensions, will be furnished by the engineer. The work shall be built in all respects according to these plans. The contractor, however, will be expected to verify the correctness of the plans, and will be required to make any changes in the work which are necessitated by errors in these plans, without extra charge, where such errors could be discovered by an impection of the plans.

#### MATERIALS

All materials shall be subject to inspection at all times during their manufacture, and the engineer and inspections shall be allowed free access to any of the works in which any portion of the material is made. Timely outer shall be given to the engineer so that his inspectors may be on hand.

Steel. The Steel used will be of two classes, viz.: High Steel, which will be used in compression members, bolsters, bearing plates, pins and rollers, and Low Steel, which will be used for tension members and rivers.

Steel may be made by the open hearth or by the Bessemer process, but no stee, shall be made at works which have not been in successio, opention for at least one year, steel made by the Clapp Griffiths process will not be accepted. All melts shall be rande from uniform stock low in phosphorus, and the manufacturer shall farinsh statisfactory evidence to the engineer that this class of material is being employed, it being understood that the furnished product is to be one in which the phosphorus does not average more than 8 too of one per cent, and never exceeds it to of one per cent.

A sample bar % of an inch in diameter shall be rolled from every melt, the method of obtaining the piece from which this sample bar is rolled shall be the same for all samples, and the amount of work on this sample bar shall be as nearly as practicable the same as on the finished product. The laboratory tests shall be made on this sample bar in its natural state without annealing. The laboratory test of High Steel made on the sample bar shall show an elastic limit of not less than 50,000 pounds per square inch, an altimate strength of not less than 50,000 pounds nor more than 90,000 pounds, an elongation of at lesst 15 per cent in eight inches and a reduced area of at lesst 35 per cent, at the point of fracture. The sample bars shall bend 180° around its own diameter without showing crack or flaw,

The laboratory tests of Low Steel made on the sample has shall show an elastic limit of not less than appropriate pressures in than 80,000 pounds, an accompanion of at least 18 per cent, in a length of eight inches, and a reduction of at least 42 per cent, at the point of fracture. In a bending test the sample has shall bend 180° and clase back against itself without showner crack or flow on the outside of the curve

The softest melts shall be selected for rivets, the only requirement as to elastic limit and ultimate strength all be that the altimate strength shall be at least 60,000 pounds per square inch

Faculties for testing sample barsshal, be furnished by the contractor at a point convenient to the steel works, and the tests shall be made at the expense of the contractor and under the direction of the Engineer. Tests may also be made from time to time on samples cut from similar plates, shapes and bars which shall show results substantially conforming to those shown by the sample tests of the same melts

All sheared edges or punched holes in steel work shall be subsequently planed or drilled out, so that none of the rough surface is ever .eft upon the work. Steel for pins shall be sound and entirely free from pining

Wrought Iron. "The iron used in tenson members shall be double refined (high test) iron; muck bass as be used at the center of the pile, but shall not constitute more than one third of the total pile. Small samples having a minimum length of eight inches, shall be furnashed by the contractor for testing, as directed by the engineer these samples shall show an elsaic hinti of at least 20,000 pounds, and an ultimate strength of at least 30,000 pounds per value; each, shall colonge at cleast 15 per cent, and show a reduced area of at least 30,000 pounds per value; each, shall colonge at cleast 15 per cent, and show a reduced area of at least 50,000 pounds per value; and the shall be of uniform fibrous character, free from crystalline anotematics.

Small samples, having a minimum length of eight inches, shall be furnished by the constractor from the from used in shapes, plates and other inseellancous forms as directed by the Engineer; these samples will show an elastic limit of at least 4,4000 pounds, and an ultimate strength of at least 4,4000 pounds per square inch, shall elongate at least to per cent. before breaking, and show a reduction of area of at least 15 per cent, at the point of fracture. In plates more than thirty inches wide an elongation of 8 per cent, and a reduction of 12 per cent, at the point of fracture will be considered satisfactory.

Cast iron shall be of the best quality of tough, grey iron.

#### RIVETED WORK

All plates, angles and channels shall be carefully straightened before they are laid out; the river holes shall be carefully spaced in truly straight lines; the river heads shall be of hemispherical pattern and the work shall be finished in a next and workman-like manner. Surfaces in contact shall be painted before they are put together. The dimensions given for rivers on the plans are the diameters of the rivers before driving.

Power riveters shall be direct acting machines, capable of exerting a yielding pressure and holding on to the rivet when the upsetting is completed.

Steel.—The several parts of each steel member shall be assembled and the holes shall be drilled, the sharp edge of the drilled hole shall be trimmed so as to make a slight filler under the rivet head, and the pueces shall be reveted together without taking apart. Should the contractors desure, the parts may be punched with holes not exceeding a 5 the diameter of the funshed hole, and this punching shall be so according to the contractors.

carate that at least  $V_{16}$  of an inch of metal is taken out all around in drilling the hole. All rivers in steel members shall be of seech the river holes shall be of such size that they will fill the hole before driving, and whenever possible the rivers shall be driven by power. All bearing surfaces shall be truly faced. The chord pieces shall be fitted together in the shop in lengths of at least five panels and marked; when so fitted there shall be no perceptible wind in the length laid out. The pair-holes shall be breed truly so as to be at exact distances, paniled with one another, and at right angles to the axes of the member.

Wrought Iron —All wrought-iron shall be panched accurately with holes ½, of an inch larger than the size of the rivet, and when put together a cold rivet shall pass through every hole without rearning. So far as possible all revers shall be driven by power. The holes for the rivets connecting the floor-bass with the posts and bisters and the stringers with the floor-beams, and, in general, the holes for all rivets which must be driven after erection, shall be accurately drilled to an iron templet. The holes for the rivets connecting the floor-beams with the posts shall be i inch in dameter, and the rivets of corresponding diameter. The pin holes in the vertical posts shall be itily parall! I with one another, and at right angles to the axes of the posts. The posts shall be stringth and free from wind

#### FORGED WORK.

The heads of the steel eye-bars shall be formed by upsetting and forging into shape by such process as may be accepted by the Engineer. No welds will be allowed. After the working is completed, the bars shall be annealed by heating them to a antoriff ander de-heat throughout their entire length and allowing them to cool slowly. The form of the beads of steel eye-bars may be modified to suit the process in use at the contractor's works, but the form of head adopted mast be such as to meet the requirements of the tests of full

The heads and the enlarged ends for screws in laterals, suspenders and counters shall be formed by upstaged by a upstaged welding process acceptable to the engineer. Welds in the body of the bar will not be allowed.

#### TESTS OF FULL-SIZED BARS

Ten full-sized stoel eye burs of sections and lengths used in the actual work shall be selected by the inspector for testing, each of these full-sized bars shall be strained till an elongation of to per cent, is obtained, and if possible broken; if broken, the fracture shall occur in the body of the bar and shall show a uniform and dictile mality of material.

The contractor will be required to furnish facilities for testing the full sized bars within a reasonable distance of his works. Should the contractor be unable to farmels such facilities, he shall be required to furnish bars at 20 per cent, larger section than those called for, without charge for the increased weight,

The fall-sared bars shall be selected from time to time as the work proceeds, the last har not to be selected. When three bars have been tested, the bars manufactured up to the time of the selection of these three test bars shall be accepted or rejected as the results of such tests, and the same shall be force given bars are tested. In the tests, the fallice of one but to develop a stretch of to per cent, before breaking, shall be sufficient reason for rejecting the whole lot, but a failure to break in the body of the bar shall not be a sufficient reason for rejecting the whole lot, but a failure to break in the body of the bar shall not be a sufficient ground for condemnation if it does not occur in more than one third of the bars stated. Should the contractor on the first attempt fail to make bars coming up to the required specifications, the engineer may order bars of 20 per cent. larger section than the plans called for, to be furnished by the contractor without charge for the increased weight.

#### MACHINE WORK

The bearing surfaces in the top chord shall be truly faced. The ends of the stringers and of the floorbeams shall be squared in a rotary facer. All surfaces so designated on the plans shall be planed. All pins shall be accurately turned to a galage, and shall be of full size throughout; pin holes shall be bored to fit the pins with a play not exceeding 1-50 of an inch. These clauses apply to all lateral connections as well as those of the main trusses. Pins shall be supplied with pilot nuts for use during erection, four for each size of vin.

All screws shall have a truncated V thread, United States standard sizes.

#### MISCELLANEOUS.

All workmanship and material, whether particularly specified or not, must be of the best kind now in use in first class bridge work. Flaws, ragged edges, surface imperfections, or irregular shapes, will be sufficient ground for ryection, rough and irregularly finished work will not be accepted.

Machine-finished surfaces shall be coated with white lead and tallow before shipment; all other parts shall be given a coat of hot boiled inseed oil.

#### TERMS

Monthly estimates will be made at the end of each month for the work done during that month. In these month, estimates, the maternal delivered at the contractor's shop, but not manifactured, shall be estimated at 5p per cent, of the contract price for finished material in Chuego, and manifactured material at 75 per cent, of the contract price for finished material in Chuego, and manifactured material at 75 per cent, of the contract price for finished material in Chicago. Payments will be made on or about the 15th day of the following month), according to those estimates, deducting from the amount of the same ten per cent, as security, to be held until the completion of the entire contract.

No material will be paid for which does not form a part of the permanent structure.

All expense of testing shall be borne by the contractor

#### TIME

The deck spans and towers shall be completed and shipped not later than January 1st, 1887. The three through spans shall be completed and shipped in February, March and June, 1887, respectively.

The railroad company may exact a penalty not exceeding \$150 per day for failure to complete the work within these specified times.

#### PROPOSALS.

Separate proposals should be made for the deck spans (including towers) and the through spans,

The prices should be by the pound at separate rates for sted, wrought roon and east tron. The processhall material dollvered on ears at Chungo. Separate proposals shall also be made for errection on the bass of finished material dollvered on ears at Chungo. Separate proposals shall also be made for errection on the bass of a single goos sum for the errection of the six deck spans (including towers, and a single goos sum for the errection of the three through spans. Excition will include setting well palates and fulling the necessary boles for anchor bolts. The contraction will be required to furnish all false work and tools of every description, and the plans of such false work shall be subject to the approach of the engineer.

The right is reserved to accept separate proposals for the deck spans and the through spans, to accept proposals for material without erection and to award the contract to other than the lowest bidder

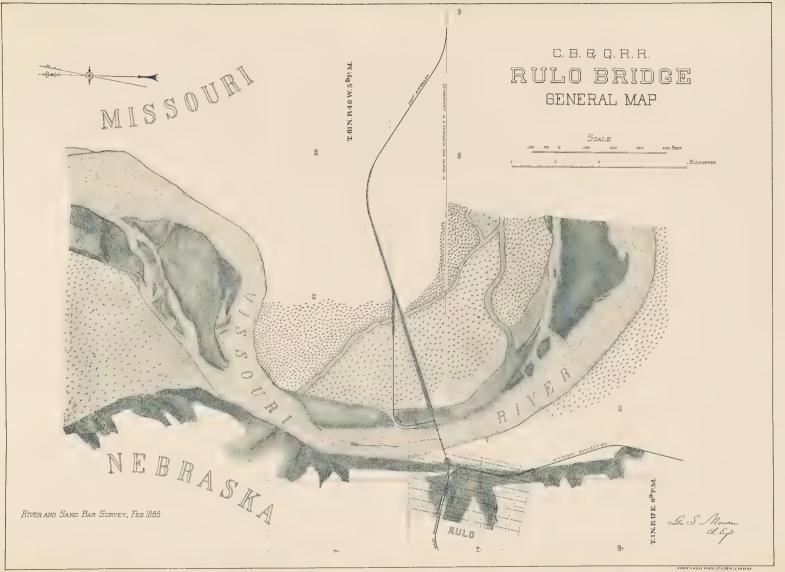
GEO, S. MORISON, Chief Engineer Bula Bridge

NEW YORK, JULY 20, 1886

### APPENDIX G.

### TESTS OF STEEL EYE BARS

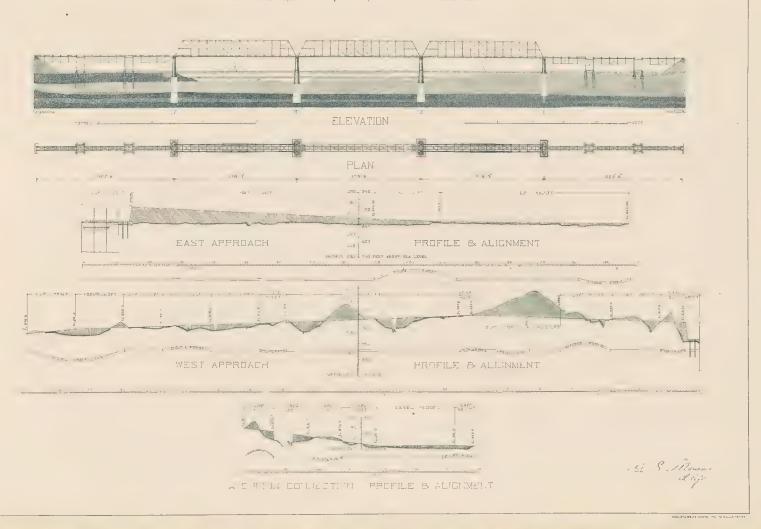
					-			_															
	TESTS ON FULL-SIZED EYE-BARS														TESTS ON SAMPLE BARS FROM SAME MELTS.								
		Drmi	INSIONS,-	-INOHES			Results of Mechanical Tests							DIAMETERS									
Original After Tes 7.								ant inston				Place of Fracture			Rel cost	Extension	Elistic Limit	Maximum Load	Per cent	Met			
Nom	nal.	Actual.			24.18		Reduction of Area Per Cent.	g agr		Elastic Lbs. per	Masin m		Organia dell's	To-mag In les		Le cent	Lastr	Lbs per	Phosphorus	Number			
W dth.	Thick ness	Length c. to c.	Wilti	Thickness.	Width.	th. Thickness.		i . h.s	Ker er	50 11	5 x 11												
7	h	300.03	6.97	0.76				264		34270	60450	Head	739	- 545	45.6	22.80	47790	74840	.090	8485			
7	1	300.03	6 97	10.1	5.51	0.74	42.08	264	13.77	35350	67220	Body	.750	-535	49.1	25.30	44140	73330	.090	8847			
7	1 1/2	300.03	6.97	1.52	5-37	1 0 87	n 90	264	14.75	31570	65262		.750	- 530	50.1	26.75	47980	75140	.051	8415			
7	1.4	300.03	6.97	1.76		-		26.1		30925	60020	Head	-743	-475	59.1	26,30	45210	71040	.086	8345			
7	t	300.03	6.97	1 01	5.46	0.64	50.36	264	13.67	35350	67800	Body	.750	- 535	49.1	25.30	44140	73330	090	8847			
7	1 1/2	300.03	6.97	1.50	6,60	1.39	12.25	264	9.01	35120	67110	**	739	.510	52 4	24 70	47570	74610	.068	8352			
7	156	424 10	6 98	1.62	> 33	0 98	54.67	396	12 11	36070	17100	fa.	740	- 545	45 8	23.00	46040	75340	.076	8339			
7	1%	424-10	7.00	1.61	5 - 57	1,00	39.70	396	11.43	33120	63340		741	.486	57.0	4,60	45460	74210	.075	8330			
7	134	424.09	6.98	1.26	5.85	1 02	33 15	396	11.74	32930	64240		-749	.550	46.1	33.00	45170	75800	.048	h+17			
5	34	422.97	5.01	0.76	4.00	0.54	43.37	396	8 65	38,40	61340		.748	.510	53 - 5	24.37	45240	76690	.062	8367			
5	34	422.72	5 00	0.77	3.70	0,48	53.87	396	9.78	42365	69325		,746	.510	53 - 3	24.80	443-90	71380	.076	8223			
7	134	300.03	7.00	1 75	5,20	1.33	47.62	264	13.37	33300	63270		.758	. 560	45 4	24.10	45200	7268o		6303			
7	34	299.61	6.97	.78	5.41	0.53	47-51	264	13.32	.0140	67530		-755	-535	49.8	23.30	466bp	74370	.08a.	9046			





### c.e.e q r.r. RULO BRIDGE

GENERAL ELEVATION, PLAN, PROFILE & ALIGNMENT





# C B. & Q. R. R. RULO BRIDGE BALL FIRE OF WATERWAYS DAWNED BIRTHAM MONTHAL FIRMS FIRMS FIRMS FRANCE TENT A 1 225 AY TAL " HE . HE



Scale of Plan RULO BRIDGE C.B. R. Q.R.R. PIER I. ,0,00

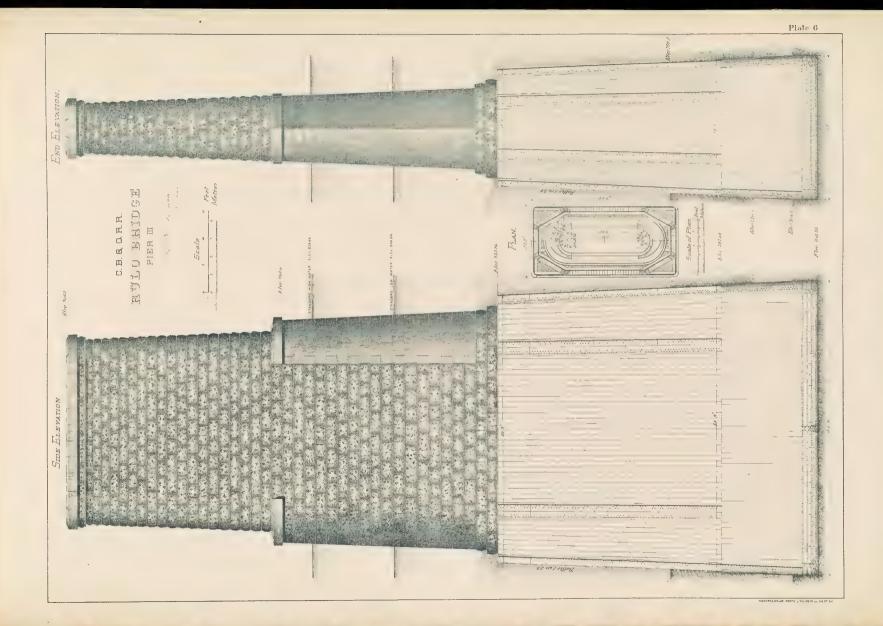
Plate 4

MOREGRA WELKE PHOTO , THE OWILL ANSTRO

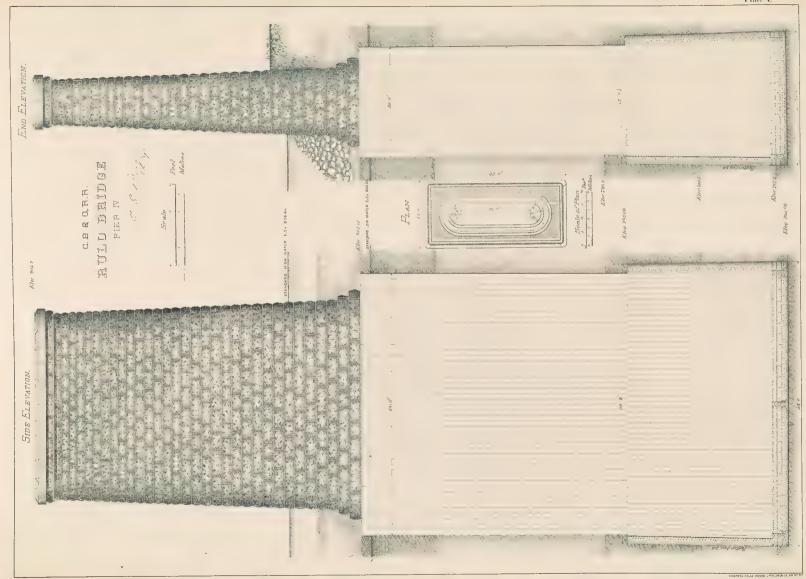


Plate 5





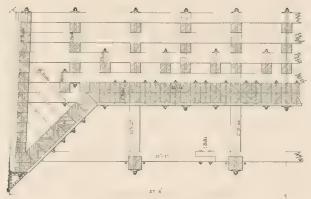




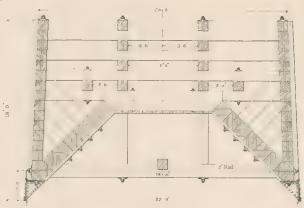


# C.B. & Q.R.R. RULO BRIDGE CAISSONS II&II

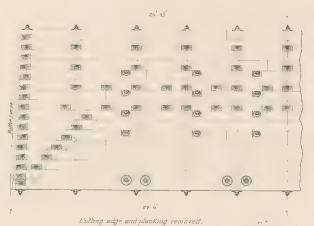
Scala



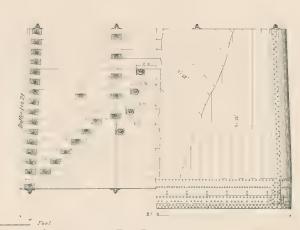
HALF LONGITUDINAL SECTION.



Cross Section.



HALF SIDE ELEVATION.



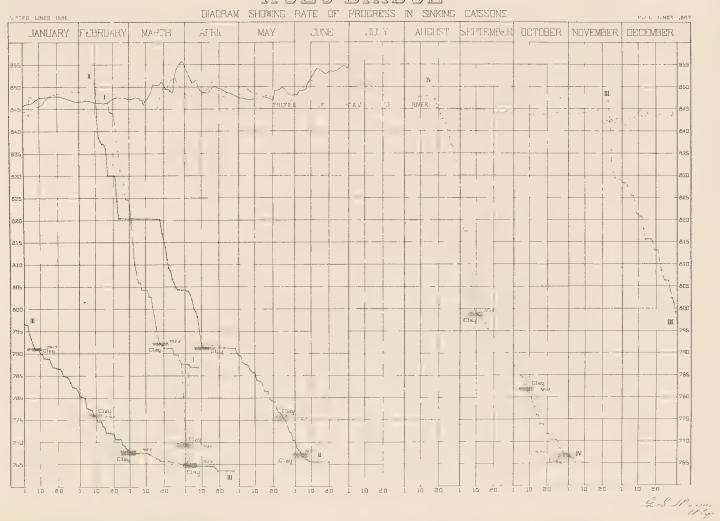
END ELEVATION.

Metres

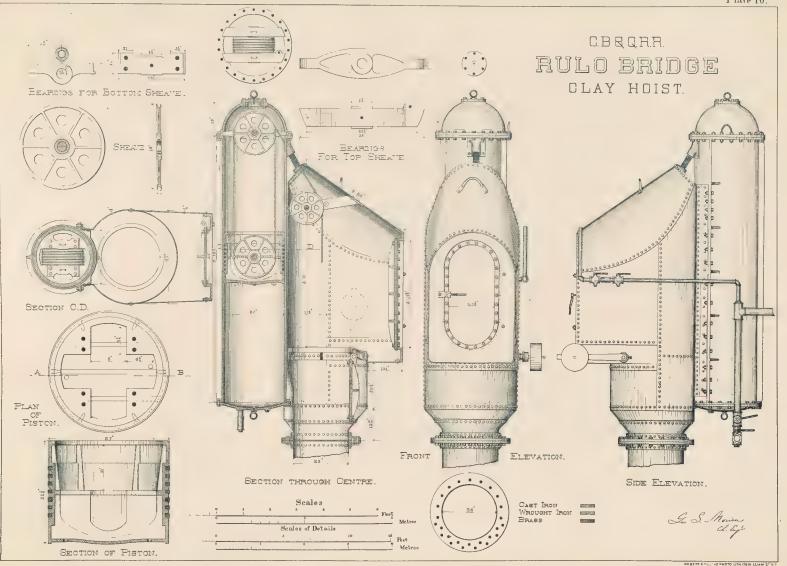
Es - Rowers



### c.s.s.q.r.r. RULD BRIDGE





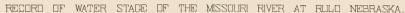




### C.B. & Q.R.R. RULO BRIDGE

La S Morson d. Ey'r

AGBERTA WELCKE PHOTO I TH PEWILL AM ST NY

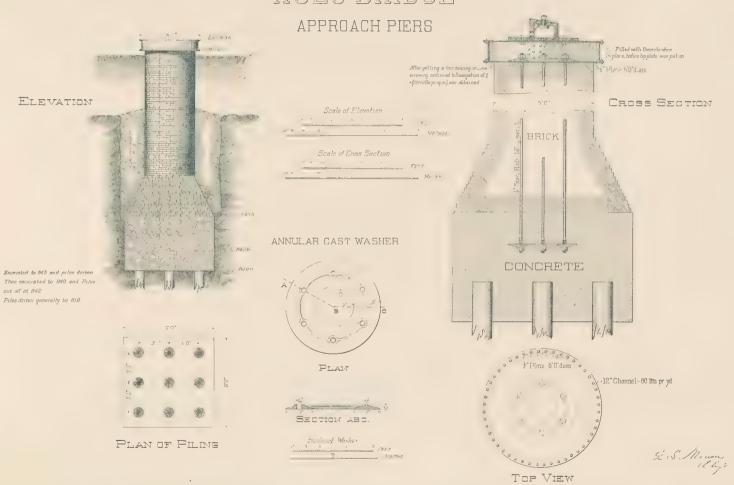






### C. B. & Q. R. R.

### RULO BRIDGE

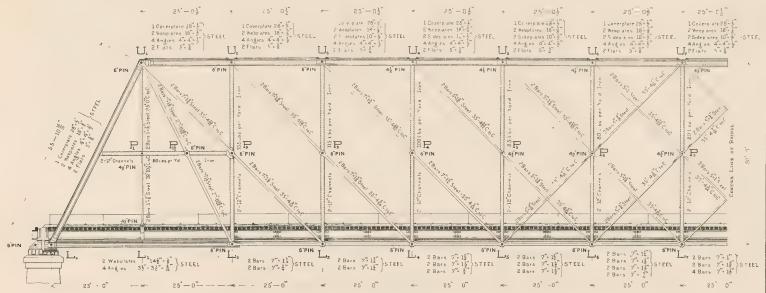




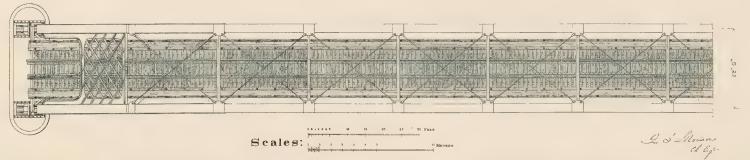
MODIFIED BUTTON STATE OF THE STATE OF THE STATE OF

# C.B. & Q. R.R. RULO BRIDGE THROUGH SPAN

#### GENERAL ELEVATION OF 375-0 SPAN



### GENERAL PLAN OF 375-0 SPAN



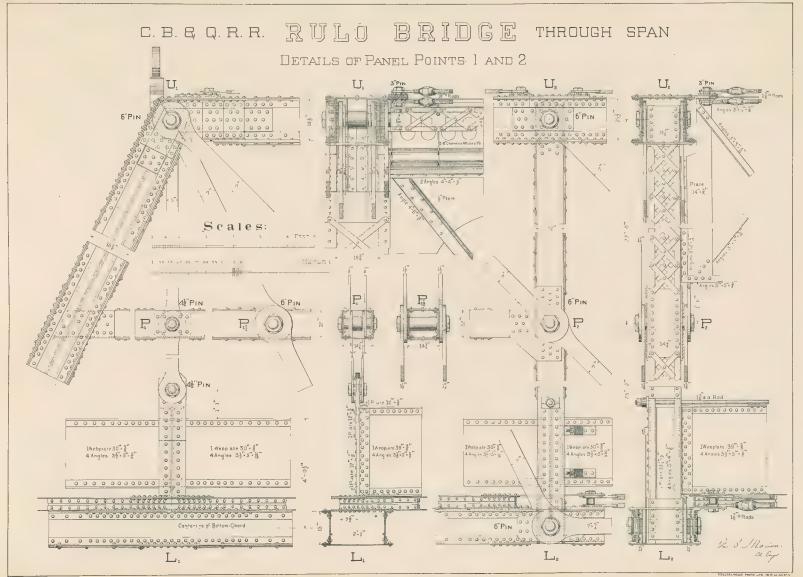


## C. B. Q C. R.R. RULO BRIDGE THROUGH SPAN

DETAILS OF 375-0 SPAN

PANEL POINT LO SIDE ELEVATION END ELEVATION FIXED END ROLLING END Inside View | 1° 10° - Outside View 1Webpale 30° 8" 4 Ai gles 3 2 5 4 2" 1 Aepp are 39" x 3" 4 Ang es 3}" x 5" > } 1 weep ate 6" 3" 4 Angles 21" 5" 3 → 217 37 → 22 37 ■ Steel Ra 5 of Pattern N=88 127 4' 3'' -\* 13"Dram 4'-9",ong PLAN Scales: La S. Nowa.





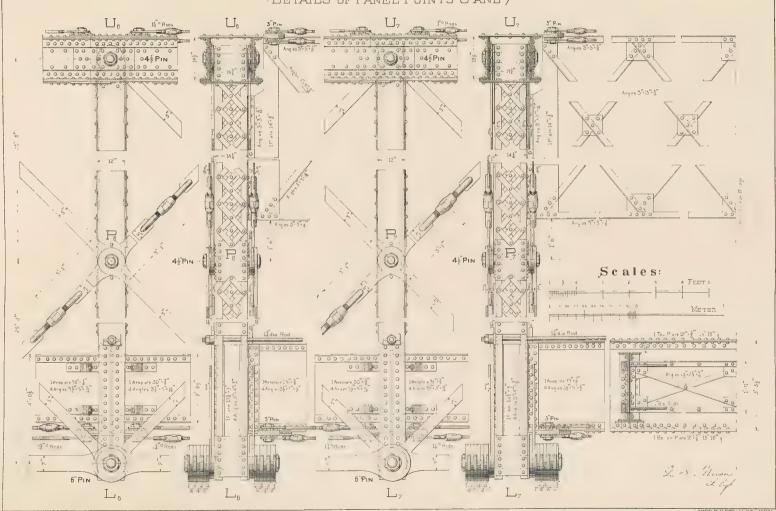


# C.B. & Q.R.R. RULO BRIDGE THROUGH SPAN DETAILS OF PANEL POINTS 3,4 AND 5.



# C.B.R.Q.R.R. RULO BRIDGE THROUGH SPAN

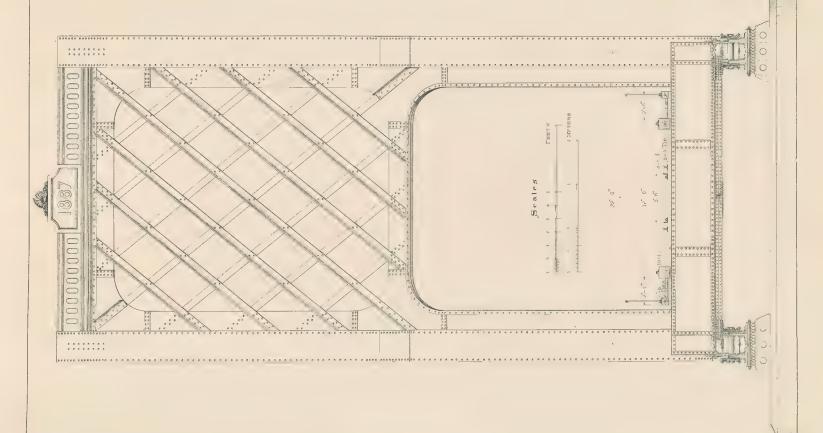
DETAILS OF PANEL POINTS 6 AND 7





# C.B.RORR. RULO BRIDGE THROUGH SPAN.

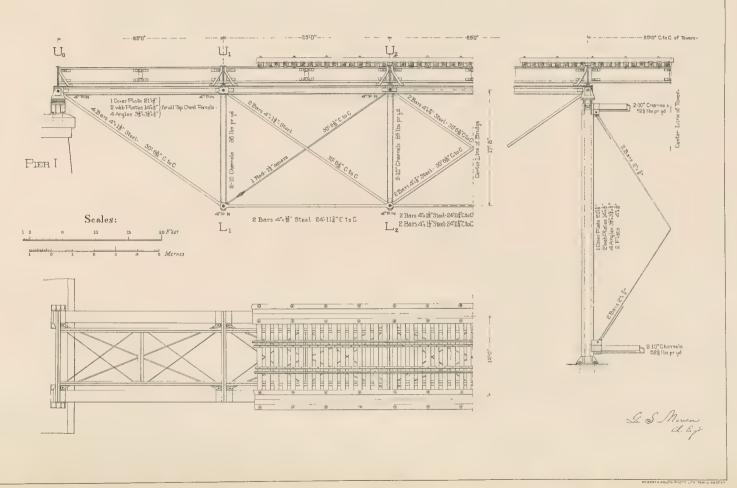
END ELEVATION.





# C.B.R.Q.R.R. RULO BRIDGE 125F DECK SPAN

GENERAL ELEVATION AND PLAN





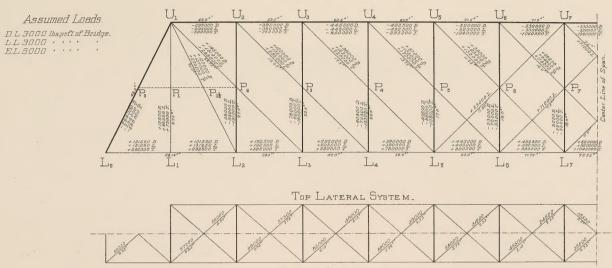
# C.B.R.Q.R.R. RULO BRIDGE 125FT DECK SPAN DETAILS OF PANEL POINTS 0, 1 AND 2 ROLLING END . Webplate 30"3" 4 Angles 32" 5"11" r - 2:2" - \* End View SIDE ELEVATION PLAN OF ROLLING END. PLAN OF BOTTOM CHORD. Scales:



## Plate 21 Center Line L. S. Morison 52} lbs.p.yd nels 52-lbsp.yd 125\*\*DECK SPAN SECTION CHOSS Seales 2-10 Channels 522 bs pyd 2 10 Channels BRIDGE \$ "Cestalquellif1S \$ "C"Cestalquellif1S 0 0 0 0 0 0 0 SUPPORTING TOWER -84-5-\$ x" 0S ahilqtavo01 \$ x" At satelqtaW S Сепјет Line 3 Pin 3"Pin OTA ELEVATION ooo 2-10 Channels 522 hs. p.yd ) I 1Webplate 30×音。 4Angles 3克×5×吊 C.B.R.Q.R.R. SIDE -45 %

## C.B. & Q.R.R. RULO BRIDGE STRAIN SHEET

### 375 THROUGH SPAN

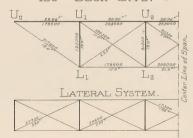


### BOTTOM LATERAL SYSTEM.

### 125 FT. DECK SPAN

5000 lbs prfl. all moving load.

500 lbs.pr.fl.wind.pressure.



L. S. Maison C. E.

